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Techniques of Water-Resources Investigations of the United States Geological Survey

Chapter Cl

FINITE-DIFFERENCE MODEL FOR AQUIFER SIMULATION IN TWO DIMENSIONS WITH RESULTS OF NUMERICAL EXPERIMENTS

By P. C. Trescott, G. F. Pinder, and S. P. Larson

Book 7

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PREFACE

The series of manuals on techniques describes procedures for planning and executing specialized work in water-resources investigations. The material is grouped under major headings called books and further subdivided into sections and chapters; section C of Book 7 is on computer programs.

"Finite-difference model for aquifer simulation in two dimensions with results of numerical experiments" supersedes the report published in 1970 entitled, "A digital model for aquifer evaluation" by G. F. Pinder as Chapter C1 of Book 7. The new Chapter C1 represents a significant improvement in the computational capability to solve the flow equations and has greater flexibility in the hydrologic situations that can be simulated.

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FINITE-DIFFERENCE MODEL FOR AQUIFER SIMULATION IN TWO DIMENSIONS WITH RESULTS OF NUMERICAL EXPERIMENTS

By P. C. Trescott, G. F. Pinder, and S. P. Larson

Abstract

The model will simulate ground-water flow in an artesian aquifer, a water-table aquifer, or a combined artesian and water-table aquifer. The aquifer may be heterogeneous and anisotropic and have irregular boundaries. The source term in the flow equation may include well discharge, constant recharge, leakage from confining beds in which the effects of storage are considered, and evapotranspiration as a linear function of depth to water.

The theoretical development includes presentation of the appropriate flow equations and derivation of the finite-difference approximations (written for a variable grid). The documentation emphasizes the numerical techniques that can be used for solving the simultaneous equations and describes the results of numerical experiments using these techniques. Of the three numerical techniques available in the model, the strongly implicit procedure, in general, requires less computer time and has fewer numerical difficulties than do the iterative alternating direction implicit procedure and line successive overrelaxation (which includes a two-dimensional correction procedure to accelerate convergence).

The documentation includes a flow chart, program listing, an example simulation, and sections on designing an aquifer model and requirements for data input. It illustrates how model results can be presented on the line printer and pen plotters with a program that utilizes the graphical display software available from the Geological Survey Computer Center Division. In addition the model includes options for reading input data from a disk and writing intermediate results on a disk.

Introduction

The finite-difference aquifer model documented in this report is designed to simulate in two dimensions the response of an aquifer to an imposed stress. The aquifer may be artesian, water table, or a combination of artesian and water table; it may be heterogeneous and anisotropic and have irregular boundaries. The model permits leakage from confining beds in which the effects of storage are considered, constant recharge, evapotranspiration as a linear function of depth to water, and well discharge. Although it was not designed for cross-sectional problems, the model has been used with some success for this type of simulation.

The aguifer simulator has evolved from Pinder's (1970) original model and modifications by Pinder (1969) and Trescott (1973). The model documented by Trescott (1973) incorporates several features described by Prickett and Lonnquist (1971) and has been applied to a variety of aquifer simulation problems by various users. The model described in this report is basically the same as the 1973 version but includes minor modifications to the logic and data input. In addition, the user may choose an equation solving scheme from among the alternating direction implicit procedure, line successive overrelaxation, and the strongly implicit procedure. The program is arranged so that other techniques for solving simultaneous equations can be coded and substituted for the iterative techniques included with the model.

The documentation is intended to be reasonably self contained, but it assumes that the user has an elementary knowledge of the physics of ground-water flow, finite-difference methods of solving partial differential equations, matrix algebra, and the FOR-TRAN IV language.

Theoretical Development

Ground-water flow equation

The partial differential equation of groundwater flow in a confined aquifer in two dimensions may be written as

$$\frac{\partial}{\partial x} (T_{xx} \frac{\partial h}{\partial x}) + \frac{\partial}{\partial x} (T_{xy} \frac{\partial h}{\partial y}) + \frac{\partial}{\partial y} (T_{yx} \frac{\partial h}{\partial x}) + \frac{\partial}{\partial y} (T_{yy} \frac{\partial h}{\partial y}) = S \frac{\partial h}{\partial t} + W(x,y,t) \quad (1)$$

in which

 $T_{xx}, T_{xy}, T_{yx}, T_{yy}$ are the components of the transmissivity tensor (L^{2t-1}) ;

h is hydraulic head (L);

- S is the storage coefficient (dimensionless);
- W(x, y, t) is the volumetric flux of recharge or withdrawal per unit surface area of the aquifer (Lt^{-1}) .

The reader is referred to Pinder and Bredehoeft (1968) for development and discussion of equation 1. In the simulation model, equation 1 is simplified by assuming that the Cartesian coordinate axes x and y are alined with the principal components of the transmissivity tensor, T_{xx} and T_{yy} , giving

$$\frac{\partial}{\partial x}(T_{xx}\frac{\partial h}{\partial x}) + \frac{\partial}{\partial y}(T_{yy}\frac{\partial h}{\partial y}) = S\frac{\partial h}{\partial t} + W(x,y,t).$$
(2)

In water-table aquifers, transmissivity is a function of head. Assuming that the coordinate axes are co-linear with the principal components of the hydraulic conductivity tensor, the flow equation may be expressed as (Bredehoeft and Pinder, 1970)

$$\frac{\partial}{\partial x}(K_{xx}b\frac{\partial h}{\partial x}) + \frac{\partial}{\partial y}(K_{yy}b\frac{\partial h}{\partial y}) = S_y\frac{\partial h}{\partial t} + W(x,y,t)$$
(3)

in which

- K_{xx}, K_{yy} are the principal components of the hydraulic conductivity tensor (Lt^{-1}) ;
- S_y is the specific yield of the aquifer (dimensionless);
- b is the saturated thickness of the aquifer (L).

Finite-difference approximations

In order to solve equation 2 or 3 for a heterogeneous aquifer with irregular boundaries, one approach is to subdivide the region into rectangular blocks in which the aquifer properties are assumed to be uniform. The continuous derivatives in equations 2 and 3 are replaced by finite-difference approximations for the derivatives at a point (the node at the center of the block). The result is N equations in N unknowns (head values at the nodes) where N is the number of blocks representing the aquifer.

Utilizing a block-centered, finite-difference grid in which variable grid spacing is permitted (fig. 1), equation 2 may be approximated as



FIGURE 1.--Index scheme for finite-difference grid and coefficients of finite-difference equation written for node (*i*, *j*).

$$\frac{1}{\Delta x_{j}} \left[\left(T_{xx} \frac{\partial h}{\partial x} \right)_{i,j+\frac{1}{2}} - \left(T_{xx} \frac{\partial h}{\partial x} \right)_{i,j-\frac{1}{2}} \right] \\
+ \frac{1}{\Delta y_{i}} \left[\left(T_{yy} \frac{\partial h}{\partial y} \right)_{i+\frac{1}{2},j} - \left(T_{yy} \frac{\partial h}{\partial y} \right)_{i-\frac{1}{2},j} \right] \\
= \frac{S_{i,j}}{\Delta t} \left(h_{i,j,k} - h_{i,j,k-1} \right) + W_{i,j,k} \tag{4}$$

in which

 Δx_i is the space increment in the x direc-

tion for column j as shown in figure 1 (L);

- Δy_i is in the space increment in the y direction for row *i* as shown in figure 1 (*L*);
- Δt is the time increment (t);
- i is the index in the y dimension;
- j is the index in the x dimension;
- k is the time index.

Equation 4 may be approximated again as

$$\frac{1}{\Delta x_{j}} \left\{ \left[T_{xx(i,j+\frac{1}{2})} \frac{(h_{i,j+1,k} - h_{i,j,k})}{\Delta x_{j+\frac{1}{2}}} \right] - \left[T_{xx(i,j-\frac{1}{2})} \frac{(h_{i,j,k} - h_{i,j-1,k})}{\Delta x_{j-\frac{1}{2}}} \right] \right\} + \frac{1}{\Delta y_{i}} \left\{ \left[T_{yy(i+\frac{1}{2},j)} \frac{(h_{i+1,j,k} - h_{i,j,k})}{\Delta y_{i+\frac{1}{2}}} \right] - \left[T_{yy(i-\frac{1}{2},j)} \frac{(h_{i,j,k} - h_{i-1,j,k})}{\Delta y_{i-\frac{1}{2}}} \right] \right\} = \frac{S_{i,j}}{\Delta t} (h_{i,j,k} - h_{i,j,k-1}) + W_{i,j,k}$$
(5)

in which

 $T_{xx(i,j+\frac{1}{2})}$ is the transmissivity between node (i,j) and node (i,j+1); $\Delta x_{j+\frac{1}{2}}$ is the distance between node (i,j) and node (i,j+1).

Equation 5 is written implicitly, that is, the head values on the left-hand side are at the new (k) time level. Following a convention similar to that introduced by Stone (1968), the notation in equation 5 may be simplified by writing

$$F_{i,j}(h_{i,j+1,k}-h_{i,j,k}) - D_{i,j}(h_{i,j,k}-h_{i,j-1,k}) + H_{i,j}(h_{i+1,j,k}-h_{i,j,k}) - B_{i,j}(h_{i,j,k}-h_{i-1,j,k}) = \frac{S_{i,j}}{\Delta t}(h_{i,j,k}-h_{i,j,k-1}) + W_{i,j,k}(6)$$

in which

Similarly.

$$B_{i,j} = \frac{\begin{bmatrix} 2T_{yy} [i,j] & T_{yy} [i-1,j] \\ \hline T_{yy} [i,j] \Delta y_{i-1} + T_{yy} [i-1,j] \Delta y_i \end{bmatrix}}{\Delta y_i}$$
(7a)

The term in brackets is the harmonic mean of

$$\frac{T_{\boldsymbol{\nu}\boldsymbol{\nu}}_{[i,j]}}{\Delta y_{i}}, \frac{T_{\boldsymbol{\nu}\boldsymbol{\nu}}_{[i-1,j]}}{\Delta y_{i-1}}$$

It represents the ratio $T_{yy_{(i-\frac{1}{2})}}/\Delta y_{i-\frac{1}{2}}$ in equation 5.

 $D_{i,j} = \frac{\left[\frac{2T_{xx[i,j]}T_{xx[i,j-1]}}{T_{xx[i,j]}\Delta x_{j-1} + T_{xx[i,j-1]}\Delta x_{j}}\right]}{\Delta x_{j}}; (7b)$ $F_{i,j} = \frac{\left[\frac{2T_{xx[i,j]}T_{xx[i,j+1]}}{T_{xx[i,j]}\Delta x_{j+1} + T_{xx[i,j+1]}\Delta x_{j}}\right]}{\Delta x_{j}}; (7c)$

$$H_{i,j} = \frac{\begin{bmatrix} 2T_{yy[i+1,j]} T_{yy[i,j]} \\ \hline T_{yy[i,j]} \Delta y_{i+1} + T_{yy[i+1,j]} \Delta y_i \end{bmatrix}}{\Delta y_i}.$$
 (7d)

Use of the harmonic mean (1) insures continuity across cell boundaries at steady state if a variable grid is used, and (2) makes the appropriate coefficients zero at no-flow boundaries.

Equation 6 is also used to approximate equation 3 by replacing S with Sy and defining the transmissivities in equations 7a through 7d as a function of the head from the preceding iteration. As an example,

$$T^n_{xx(i,j)} = K_{xx(i,j)} b^{n-1}_{i,j,k}$$

in which n is the iteration index.

The notation may be simplified further by omitting subscripts not including a "+1" or "-1" (except where necessary for clarity) and by following the convention that unknown terms are placed on the left-hand side of the equations. Equation 6 may be rearranged and expressed as

$$Bh_{i-1} + Dh_{j-1} + Eh + Fh_{j+1} + Hh_{i+1} = Q$$
 (8)
in which

$$E = -(B+D+F+H+\frac{S}{\Delta t});$$
$$Q = -\frac{S}{\Delta t}h_{k-1} + W.$$

Source term

The source term W(x,y,t) can include well discharge, transient leakage from a confining bed, recharge from precipitation and evapotranspiration. In the model the source term is computed as

$$W_{j,j,k} = \frac{Q_{w[i,j,k]}}{\Delta x_j \ \Delta y_i} - q_{ro[i,j,k]} - q'_{i,j,k} + q_{et[i,j,k]}$$

in which

 $Q_{w[i,j,k]}$ is the well discharge $(L^{3}t^{-1})$;

 $q_{rc[i,j,k]}$ is the recharge flux per unit area (Lt^{-1}) ;

 $q'_{i,j,k}$ is the flux per unit area from a confining layer (Lt^{-1}) ;

 $q_{et[i,j,k]}$ is the evapotranspiration flux per unit area (Lt^{-1}) .

Leakage

Leakage from a confining layer or streambed in which storage is considered may be approximated by

$$q'_{i,j,k} \approx (h_{i,j,0} - h_{i,j,k}) \left(\frac{K'_{i,j}}{3m_{i,j}^2 S_{s[i,j]}} \right)^{\frac{1}{2}} m_{i,j} \cdot \left\{ 1 + 2\sum_{n=1}^{\infty} \exp\left[\frac{-n^2}{\left(\frac{K'_{i,j}t}{3m_{i,j}^2 S_{s[i,j]}} \right)} \right] \right\} + \frac{K'_{i,j}}{m_{i,j}} (\hat{h}_{i,j,0} - h_{i,j,0})$$
(9)

in which

 $h_{i,j,0}$ is the hydraulic head in the aquifer at the start of the pumping period (L); $\hat{h}_{i,j,0}$ is the hydraulic head on the other side of the confining bed (L); K'_{i} is the hydraulic conductivity of the confining bed (L/t): is the thickness of the $m_{i,j}$ confining bed (L); $S_{s[i,j]}$ is the specific storage in the confining layer (L^{-1}) ; $(K'_{i,j}t/m_{i,j}^2S_{s[i,j]})$ is dimensionless time; see Bredehoeft and Pinder (1970) for a discussion of leakage versus dimensionless time: t is the elapsed time of the pumping period (t).

Equation 9 is modified from Bredehoeft and Pinder (1970, p. 887); note that it is the sum of two terms; the first term on the righthand side of equation 9 considers transient effects; the second term is steady leakage due to the initial gradient across the confining bed. (See fig. 2.) Figure 2 illustrates the head distribution in the confining layer at any given point in the aquifer system at two different times in each of two successive pumping periods. (The succession of head values in the aquifer is shown by $h_{i,j,1}, \ldots$ $h_{i,i,4}$) The solid line represents the head distribution at the beginning of the pumping period; the gradient $((h_{i,j,0}-h_{i,j,0})/m_{i,j})$ appears in the second term of equation 9. The hatchured line represents the head distribution in the confining bed after stressing the pumped aquifer and is a summation of the initial head distribution and the change in head distribution due to the stresses on the aquifer. The factor T_L in figure 2 represents the part of the first term in equation 9 independent of head (that is, the transient leakage coefficient).

In figure 2a the confining bed is assumed to have significant storage, pumping has lowered the head to $h_{i,j,1}$ and the net (or total) gradient is for some dimensionless time <0.5. After transient effects have dissipated, a uniform gradient across the confining bed is established. (See fig. 2b.) Then if the stress on the aquifer is changed by turning off pumping wells and starting recharge wells, the initial head distribution in the confining bed







FIGURE 2.—In the first pumping period, (a) illustrates the head distribution in the confining bed at one time when transient leakage effects are significant; (b) illustrates a time after transient effects have dissipated; in the second pumping period, (c) is analogous to (a) and (d) is analogous to (b).

for the new conditions is shown in figure 2c and is equal to the final distribution for the first pumping period. The net head distribution in figure 2c is affected by storage in the confining bed and is for some dimensionless time <0.5 (in the second pumping period). After storage effects have dissipated, the net gradient is shown in figure 2d.

For a simulation of several pumping periods, the program assumes that transient leakage effects from previous pumping periods have dissipated. This is accomplished at the start of each pumping period by initializing $h_{i,i,0}$ to the head at the end of the previous pumping period and setting t (and thereby dimensionless time) to zero (note that the parameter storing the cumulative simulation time is not affected). The assumption is reasonable if dimensionless time for previous pumping periods is at least 0.5 (Bredehoeft and Pinder, 1970, fig. 4) and can be checked by noting the value of dimensionless time printed in the output for the end of the previous pumping period. If the assumption is not valid, the code will need to be modified to include transient effects for one or more previous pumping periods.

In the model, equation 9 is used until dimensionless time reaches 3×10^{-3} ; otherwise, the equation

$$q'_{i,j,k} \cong (h_{i,j,0} - h_{i,j,k}) \frac{K'_{i,j}}{m_{i,j}} \left\{ 1 + 2 \sum_{n=1}^{\infty} \exp\left[-n^2 \pi^2 \left(\frac{K'_{i,j}t}{3m_{i,j}^2 S_{s[i,j]}} \right) \right] \right\} + \frac{K'_{i,j}}{m_{i,j}} (\hat{h}_{i,j,0} - h_{i,j,0}) \quad (10)$$

is used. Equation 10 is computationally more efficient for dimensionless times greater than about 3×10^{-3} .

The transient parts of equations 9 and 10 are based on the analytic solutions for the flux from a confining layer resulting from an instantaneous stepwise change in head in the aquifer. The factor of 1/3 appearing in dimensionless time is included in order to approximate the transient flux resulting from the actual drawdown in the aquifer. In effect the transient flux is approximated by applying a step change in head equal to the drawdown from the start of the pumping period at 1/3 of the elapsed time in the pumping period. (See fig. 3.)

The results of several numerical experiments indicate that it would be better to use



FIGURE 3.—The total drawdown at the elapsed time, t, in the pumping period (a) is applied at t/3 in equations , 9 and 10 to approximate $q^*_{i,j,k}$, the transient part of $q'_{i,j,k}$ (b).



FIGURE 4.—Comparison of analytic solution and numerical results using factors of 2 and 3 in the transient leakage approximation.

a factor of 1/3 rather than the factor of 1/2 used in the approximation by Bredehoeft and Pinder (1970). In figure 4 are plotted numerical results and Hantush's (1960) analytic solution for $\beta = 0.021$ ($\beta = 0.25 \ r \ [K'S_s/$ TS]^{1/2} and r is the radial distance from the center of the pumping well). The drawdown values using a factor of 1/3 are below but very close to the analytic curve after the first few time steps. The results using a factor of 1/2 are close to the analytic solution but are about twice as far above the analytic curve as the factor of 1/3 results are below the curve. In figure 5 are plotted the percent difference between the volume of leakage computed numerically and the volume determined analytically. Two sets of data are shown: a 14-step simulation between dimensionless times of 10^{-5} and 5.8×10^{-2} and an 11-step simulation between dimensionless times of 5.8×10^{-3} and 4.4×10^{-1} . Based on those experiments, if 4 or 5 time steps are simulated before the period of interest, the volume of leakage and the drawdown computed numerically using a factor 1/3 in equations 9 and 10 are close to the analytic solution.

Evapotranspiration

Evapotranspiration as a linear function of depth below the land surface is computed as

$$q_{et[i,j,k]} = \begin{cases} Q_{et} & [h_{i,j,k} \ge G_{i,j}] \\ Q_{et} - \frac{Q_{et}}{ET_z} (G_{i,j} - h_{i,j,k}) \ [ET_z > (G_{i,j} - h_{i,j,k}) \ ; \ h_{i,j,k} < G_{i,j}] \\ 0 & [ET_z \le (G_{i,j} - h_{i,j,k})] \end{cases}$$
(11)



FIGURE 5.—Percent difference between the volume of leakage computed with the model approximation and Hantush's analytical results.

in which

- Q_{et} is the maximum evapotranspiration rate (Lt^{-1}) ;
- ET_z is the depth below land surface at which evapotranspiration ceases (L);
- $G_{i,j}$ is the elevation of the land surface (L).

This relationship (illustrated in fig. 6) is treated implicitly by separating the equation into two terms ¹: one term is included with the E coefficient on the left-hand side of equation 8; the other is a known term included in Q on the right-hand side of equation 8.

Other functions for evapotranspiration can be defined (for example, decreasing exponentially with depth), but it may be more difficult to treat these relationships numerically. The easiest approach is to make evapotranspiration an explicit function of the head at the previous iteration, but this may cause oscillations and difficulties with convergence. Normally, the oscillations may be dampened by making evapotranspiration a function of the head for the two previous iterations. A more sophisticated approach is to use the Newton-Raphson method, which is a rapidly converging iterative technique for treating systems of non-linear equations. (See, for example, Carnahan, Luther, and Wilkes, 1969, p. 319–329.)

Computation of head at the radius of a pumping well

The hydraulic head computed for a well node represents an average hydraulic head

¹ Some of the methods for implicit treatment of evapotranspiration, storage, and leakage have been adapted from Prickett and Lonnquist (1971).



FIGURE 6.—Evapotranspiration decreases linearly from Q_{et} where the water table is at land surface to zero where the water table is less than or equal to $G_{i,j} = ET_{x}$.

computed for the block and is not the head in a well. An option to compute the head and drawdown at a well is included in the model. This computation uses the radius, r_e , of a hypothetical well for which the average value of head for the cell applies. An approximating equation is then used to make the extrapolation from r_e to the radius of a real well.

The radius r_e can be computed as (Prickett, 1967)

$$r_e = r_1/4.81$$
 (12)

in which $r_1 = \Delta x_j = \Delta y_i$ (fig. 7). Equation 12 assumes steady flow, no source term other than well discharge in the well block, and that the area around the well is isotropic and homogeneous. The derivation of equation 12 can be seen with reference to figure 7 in which the four nodes adjacent to node i,j are assumed to have head values equal to the value at node i-1, j. In figure 7a one-quarter of the discharge to the well node i,j is computed by the model as



FIGURE 7.—Flow from cell (i-1,j) to cell (i,j)(a) and equivalent radial flow to well (i,j) with radius $r_{a}(b)$.

$$\frac{Q_{w[i,j,k]}}{4} = \Delta x_j T_{i,j} \frac{\Delta h}{\Delta y}$$
(13)

in which

 $\Delta h = h_{i-1,j,k} - h_{i,j,k};$ $T_{i,j} = T_{xx[i,j]} = T_{yy[i,j]}.$

The equivalent discharge for radial flow to the well is given by the Thiem (1906) equation expressed as (see fig. 7b)

$$\frac{Q_{w[i,j,k]}}{4} = \frac{\pi T_{i,j}}{2} \frac{\Delta h}{\ln(r_1/r_e)}.$$
 (14)

Equating the discharges in equations 13 and 14 gives equation 12.

The Thiem equation is commonly used to extrapolate from the average hydraulic head for the cell at radius r_e to the head, h_w , at the desired well radius, r_w (Prickett and Lonnquist, 1971; Akbar, Arnold, and Harvey, 1974) and is written in the form

$$h_w = h_{i,j,k} - \frac{Q_{w[i,j,k]}}{2\pi T_{i,j}} \ln (r_e/r_w).$$
 (15)

Equation 15 assumes that: (1) flow is within a square well block and can be described by a steady-state equation with no source term except for the well discharge, (2) the aquifer is isotropic and homogeneous in the well block, (3) only one well is in the block and it fully penetrates the aquifer, (4) flow is laminar, and (5) well loss is negligible.

In an unconfined aquifer, the analogous equation is

$$H_{w} = \sqrt{H_{i,j,k}^{2} - \frac{Q_{w[i,j,k]}}{\pi K_{i,j}} \ln(r_{c}/r_{w})} \quad (16)$$

in which

$$H_{i,j,k} = h_{i,j,k} - \text{BOTTOM}$$
 (I,J) is the saturated thickness of the aquifer at radius r_e (L);
 H_w is the saturated thick-

 H_w is the saturated thickness of the aquifer at the well (L);

 $K_{i,j} = K_{xx[i,j]} = K_{yy[i,j]};$

BOTTOM (I,J) = elevation of the bottom of the aquifer (The uppercase letters indicate that this parameter is identical to that used in the model.)

When the saturated thickness computed with equation 16 is negative, the message, 'X,Y WELL IS DRY' is generated. This situation has no effect on the computations, but should stimulate careful consideration of the value of results for subsequent time steps in the simulation.

The conditions when the Thiem equation or equation 16 will be accurate can be computed. Table 1 was prepared to give a few examples of the head values computed by the model with the Thiem equation for a well with a radius of 1.25 feet in an infinite leaky artesian aguifer and in an infinite nonleaky artesian aquifer. The analytic solutions for these conditions are included for comparison. A variable grid was used in the model but the dimensions of the well block were $\Delta x = \Delta y$ =1,000 feet. For conditions which depart significantly from the assumptions given above (for example, a well in a rectangular block with anisotropic transmissivity or a well in a large block that has a significant amount of leakage) the results using equations 15 and 16 should be checked with a more rigorous analysis. Additional drawdown due to the effects of partial penetration and well loss can be computed separately or added to the code as needed.

Table 1.—Comparison of drawdowns computed with equation 15 and the analytic values

	Time step	Dimen-	Drawdown		
Aquifer		less time	Approxi- mation	Analytic	
Nonleaky		Tt/r^2S			
artesian	3	3.0×10^{5}	41.1	42.7	
Leaky	14	3.7×10^7 K't/m ² S.	58.3	58.1	
artesian	3	0.028	51.8	52.1	
	9	.44	57.1	57.3	

Combined artesian-water-table simulation

Simulation of an aquifer that is partly confined and elsewhere has a free surface requires special computations for the transmissivity, storage coefficient, and leakage term. The following paragraphs describe the computations required. Some of the methods of coding these procedures have been adapted from Prickett and Lonnquist (1971).

Transmissivity

The transmissivity is computed as the saturated thickness of the aquifer times the hydraulic conductivity. This computation requires that the elevations of the top and bottom of the aquifer be specified. Where the aquifer crops out, the top of the aquifer is assigned a fictitious value greater than or equal to the elevation of the land surface.

Storage

The storage term requires special treatment at nodes where a conversion from artesian to water-table conditions, or vice versa, occurs during a time step. The program first checks for a change at a node during the last iteration. If there has been a change from artesian to water-table conditions, the storage term is

$$\frac{S_{y[i,j]}}{\Delta t}(h_{i,j,k}^n - h_{i,j,k-1}) - \text{SUBS}$$

in which

SUBS =
$$(h_{i,j,k-1} - \text{TOP}(\mathbf{I}, \mathbf{J}))$$

 $(S_{i,j} - S_{u[i,j]}) / \Delta t;$

The purpose of SUBS is to correctly apportion the storage coefficient and specific yield according to the relationship in figure 8a.

For a change from water-table to artesian conditions, the storage term is

$$\frac{\delta_{i,j}}{\Delta t} (h_{i,j,k}^n - h_{i,j,k-1}) - \text{SUBS}$$

in which

$$SUBS = (h_{i,j,k-1} - TOP(I,J)) \quad (S_{y[i,j]} - S_{i,j}) / \Delta t.$$

SUBS subtracts the storage coefficient and adds the specific yield for the distance B illustrated in figure 8b.

Leakage

To treat leakage more realistically if parts of an artesian aquifer change to water-table conditions, the maximum head difference across the confining bed is limited to $\hat{h}_{i,j,0}$ -TOP (I,J).

Two examples illustrate the calculation of leakage in conversion simulations. In figure 9a the head at the start of the pumping period, $h_{i,j,0}$ is below the water-table head, $\hat{h}_{i,j,0}$, but above the top of the aquifer; the current pumping level is below the top of the aquifer. The applicable equation is



FIGURE 8.—Storage adjustment is applied to distance A in conversion from artesian to water-table conditions (a) and to distance B in conversion from water-table to artesian conditions (b).



FIGURE 9.-Two of the possible situations in which leakage is restricted in artesian-water-table simulations.

$$q'_{i,j,k} = \frac{K'_{i,j}}{\tilde{m}_{i,j}} (\hat{h}_{i,j,0} - h_{i,j,0}) + T_L (h_{i,j,0} - \text{TOP}(I,J)).$$

For this situation $q'_{i,j,k}$ appears on the righthand side of the difference equation and is treated explicitly. Only if both $h_{i,j,o}$ and $h^n_{i,j,k}$ are above the top of the aquifer is the leakage term treated implicitly by including T_L in the E coefficient. This is accomplished in the code by setting U=1.

In the second example (fig. 9b), both $h_{i,j,0}$ and $h^n_{i,j,k}$ are below the top of the aquifer and the equation for leakage reduces to

$$q'_{i,j,k} = \frac{K'_{i,j}}{m_{i,j}} (\hat{h}_{i,j,0} - \text{TOP}(I,J))$$

If leakage across a subjacent confining bed is significant, it will be necessary to add a second leakage term. The flux described by this term will not be restricted where watertable conditions occur.

Test Problems

In a subsequent section the computational work required for solution of four test problems by the numerical techniques available in the model is analyzed. It is appropriate, however, to introduce the test problems here because they are used in the discussion of iteration parameters in the section on numerical techniques. The problems are for steady-state conditions since the resulting set of simultaneous equations are more difficult to solve than are the set of equations for transient problems which generally involve smaller head changes.

For each of these problems a closure criterion was chosen to decide when a solution is obtained to the set of finite-difference equations. (See Remson, Hornberger, and Molz, 1971, p. 185–186.) Normally, in this model, a solution is assumed if:

$$\operatorname{Max} | h^n - h^{n-1} | \leq \varepsilon$$

where ε is an arbitrary closure criterion (L). For the purpose of the numerical comparisons given later in this documentation, the absolute value of the maximum residual (defined by equation 28) is used to compare methods.

The first problem is a square aquifer with uniform properties and grid spacing (fig. 10). The finite-different grid is 20×20 , but only 18 rows and columns are inside the aquifer because the model requires that the first and last rows and columns be outside the aquifer boundaries. Two discharging wells and one recharge well are the stress on the system; boundaries are no flux except for part of one side which is a constant-head

PROBLEM CHARACTERISTICS

Transmissivity: Txx = Tyy = 0.1 ft 2 /s (0.009 m 2 /s) Grid spacing: $\Delta x = \Delta y = 5000$ ft (1500 m) Dimensions of grid. 18×18



EXPLANATION OF SYMBOLS

 ✓ Constant head boundary, elevation 0 ft (0 m)
 //////// No-flow boundary
 W Discharging well at 2 ft ³/s (0.06 m ³/s)
 R Recharging well at 2 ft ³/s (0.06 m ³/s)
 -5- Line of equal drawdown Interval 5 ft. (15 m)

FIGURE 10.—Characteristics of test problem 1.

boundary. A closure criterion of 0.001 foot (0.0003 metre) was used.

Konikow (1974) designed the second problem in his analysis of ground-water pollution at the Rocky Mountain Arsenal northeast of Denver, Colo. It is included as one of the test problems because it is typical of many field problems and because there is some difficulty in obtaining a steady-state solution with the alternating-direction implicit procedure. The transmissivity distribution is shown in figure 11; note the extensive areas where the transmissivity is zero because the surficial deposits are unsaturated. The finite-difference grid representing this aquifer is 25×38 with square blocks 1,000 feet (300 metres) on a side. The model has constant-head boundaries at the South Platte River and where the aquifer extends beyond the limits of the model; elsewhere no-flux boundaries are employed. Although this is a water-table aquifer, it is assumed for problem 2 that transmissivity is independent of head. The model includes 49 irrigation wells and recharge from canals and irrigation. In figure 11 the observed water-table configuration is shown, and it is used as the initial surface for the simulation; the computed water table is generally within a few feet of the observed. For this problem the closure criterion is 0.001 foot (0.0003 metre).

The third problem is a cross-section with three horizontal layers and other characteristics shown in figure 12. Transmissivity equals hydraulic conductivity for this problem because it is conceived as a slice one unit wide. The values for transmissivity are arbitrary. Note in particular that the horizontal conductivity is 100 times the vertical conductivity in all layers and that the middle layer acts as a confining layer between the upper and lower layers. The coefficients $B_{i,j}$ and $H_{i,j}$, however, are 100 times greater than the horizontal coefficients $D_{i,j}$ and $F_{i,j}$ because of



FIGURE 11.—Transmissivity and observed water-table configuration for test problem 2 (fieldwork and model design by Konikow, 1975).

the grid spacing used. For this problem, the closure criterion is 0.0001 feet (0.00003 metre).

In the third problem the upper boundary (the water table) is fixed as a constant-head boundary. It could also be treated as a no-flow boundary which would effectively confine the system. This model was not designed specifically for simulation of cross sections, and consequently it does not have provision for a moving boundary. Rather than modifying this one-phase model for a moving-boundary problem, it would be better to design a model specifically for this purpose. The two-phase model described by Freeze (1971) is a good example. The fourth problem is to consider the water-table case of the second problem. The only difference from problem 2 is that transmissivity is dependent upon (1) head in the aquifer, (2) aquifer base elevation, and (3) hydraulic conductivity of the aquifer.

Numerical Solution

In Pinder (1969) and Trescott (1973) the iterative, alternating-direction implicit procedure (ADI) was the only option available for numerical solution. For many field problems ADI is convergent and competitive, in terms of the computational work required,



FIGURE 12 .--- Characteristics of test problem 3.

with other iterative techniques available. It may be difficult, however, to obtain a solution for some problems with ADI (for example, steady-state simulations involving extremely variable coefficients). Consequently, it is convenient to have available other numerical techniques that may be more suited than ADI to particular problems. The three numerical methods available with this model are ADI, the strongly implicit procedure (SIP), and line successive overrelaxation (LSOR).

The following sections outline the computational algorithms for the three numerical methods. More details are given in the discussion on SIP, because that method is more complex.

For additional details on the theory behind the methods and rigorous analysis of convergence rates, see for example, Varga (1962) and Remson, Hornberger, and Molz (1971). The methods are presented in order of increasing complexity. In general, the more complex methods converge more rapidly and are applicable to more types of problems than the simpler methods such as LSOR. For clarity, the numerical treatment of the source term is left to other sections.

Line successive overrelaxation

Line successive overrelaxation (LSOR) improves head values one row (or column) at a time. Whether the solution is oriented along rows or columns is generally immaterial for isotropic problems but has a significant affect on the convergence rate in anisotropic problems. The solution should be oriented in the direction of the larger coefficients, either $B_{i,j}$ and $H_{i,j}$ or $D_{i,j}$ and $F_{i,j}$ (Breitenbach, Thurnau, and van Poollen, 1969, p. 159). Differences in the magnitude of the coefficients may result from anisotropic transmissivity or from a large difference in grid spacing between the x and y directions. In problem 3 the largest transmissivity is in the horizontal direction in each layer, but the small grid spacing in the vertical direction makes the coefficients $B_{i,i}$ and $H_{i,i} \gg D_{i,i}$ and $F_{i,j}$.

With the solution oriented along rows, an

intermediate value is computed by the line Gauss-Seidel iteration formula,

$$Dh_{j-1}^{\dagger}+Eh^{\dagger}+Fh_{j+1}^{\dagger}=Q_{\lambda},\,j=1,2,\ldots,N_{x}$$
 (17a)
in which

$$Q_{\lambda} = W - Bh_{i-1}^{n} - Hh_{i+1}^{n-1} - \frac{S}{\Delta t}h_{k-1};$$

 h^{\dagger} is the intermediate head value at node (i,j);

 N_x is the number of nodes in a row.

Equation 17a can be expressed in matrix form as

$$\bar{\bar{A}}_{\lambda}\bar{h}^{\dagger} = \bar{Q}_{\lambda}.$$
 (17b)

In order to reduce rounding errors, equation 17b is put in residual form. (See Wein-



FIGURE 13.—Hypothetical problem with 9 interior nodes.

 $h = H_0$

Boundary conditions are not included in this equation because they are treated in the model without adding or subtracting terms to \bar{R}_{λ}^{n-1} .

stein, Stone, and Kwan, 1969, p. 283, and Breitenbach, Thurnau, and van Poollen, 1969, p. 159.) This is accomplished by adding and subtracting $\bar{A}_{\lambda}\bar{h}^{n-1}$ to the right-hand side of equation 17b giving

$$\bar{\bar{A}}_{\lambda}\bar{h}^{\dagger} = \bar{Q}_{\lambda} + \bar{\bar{A}}_{\lambda}\bar{h}^{n-1} - \bar{\bar{A}}_{\lambda}\bar{h}^{n-1}.$$
(17c)

Rearrange equation 17c to read

$$\bar{\bar{A}}_{\lambda}\bar{\xi}^{\dagger} = \bar{R}_{\lambda}^{n-1} \tag{17d}$$

in which

$$ar{\xi}^{\dagger} = ar{h}^{\dagger} - ar{h}^{n-1};$$

 $ar{R}_{\lambda}^{n-1} = ar{Q}_{\lambda} - ar{A}_{\lambda} ar{h}^{n-1}.$

Equation 17d is the LSOR residual formulation and expanded has the following form for a 3×3 problem (fig. 13):

$egin{array}{c} 0 \ E_7 \ D_8 \end{array}$	$F_7 E_8 D_9$	$\cdot F_{s} E_{s}$	+1+2+3+4+5+6+7+8+9 	_	$\begin{array}{c} R \stackrel{n-1}{_{\lambda 1}} \\ R \stackrel{n-1}{_{\lambda 2}} \\ R \stackrel{n-1}{_{\lambda 2}} \\ R \stackrel{n-1}{_{\lambda 3}} \\ R \stackrel{n-1}{_{\lambda 4}} \\ R \stackrel{n-1}{_{\lambda 5}} \\ R \stackrel{n-1}{_{\lambda 6}} \\ R \stackrel{n-1}{_{\lambda 7}} \\ R \stackrel{n-1}{_{\lambda 9}} \end{array}$
	D_9	<i>E</i> ,	59 		$K_{\lambda 9}^{n-1}$

The first row is solved by the Thomas algorithm for simultaneous equations with a tridiagonal coefficient matrix. The Thomas algorithm is given in many references. (For example, see Pinder and Bredehoeft, 1968; von Rosenberg, 1969; Remson, Hornberger, and Molz, 1971.) It is outlined below for equation 17 using notation from the program code (The coefficients D,E,F, and the known term \bar{R}^{n-1} have been subscripted with [i,j] for clarity). BE_j is an intermediate coefficient.

Recognizing that

$$D_{i,1} = F_{i,N_x} = 0,$$

an intermediate vector \overline{G} is computed by forward substitution as

$$W = E_{i,j} - D_{i,j} (BE_{j-1}),$$

$$BE_{j} = F_{i,j} / W$$

$$G_{j} = (R^{n-1}_{i,j} - D_{i,j} (G_{j-1})) / W$$

The values of $\bar{\xi}^{\dagger}$ for row *i* are then computed by backward substitution as

$$\xi_{i,j,k}^{1} = G_{j} - BE_{j}\xi_{i,j+1,k}^{\dagger}$$

where

$$\xi^{\dagger}_{\imath,N_x,k} = G_{N_x}$$

since

$$BE_{N_{\pi}}=0.$$

The head values for row 1 are then computed by the equation

$$h_{i,j}^{n} = h_{i,j}^{n-1} + \omega \xi_{i,j}^{\dagger}, j = 1, \dots, N_{x}$$

If ω is 1, the solution is by the line Gauss-Seidel formula, but convergence is slow in general. The convergence rate is improved significantly by "overrelaxation" with $1 < \omega < 2$. Discussion of the acceleration parameter is deferred until after the following section on two-dimensional correction.

Two-dimensional correction to LSOR

In certain problems, the rate of convergence of LSOR can be improved by applying a one-dimensional correction (1DC) procedure introduced by Watts (1971) or the extended two-dimensional correction (2DC) method described by Aziz and Settari (1972). These methods remove the components of certain eigenvectors in the LSOR iteration matrix from the solution vector. If the eigenvalues associated with these eigenvectors dominate the problem, particularly those including anisotropy, the convergence rate is greatly improved.

The 2DC method is applied after one or more LSOR iterations. The corrected head values are used as an improved starting point for the next iteration and the process is repeated until convergence is achieved.

The two-dimensional correction for the head at (i,j) is defined as

$$h_{i,j,k}^{n^*} = h_{i,j,k}^n + \alpha_i + \hat{\beta}_j, \quad i = 1, \dots, N_y$$

 $j = 1, \dots, N_x$

in which

 $h_{i,j,k}^{n^*}$ is the corrected head at iteration *n*;

 α_i is the correction for row *i*;

 $\hat{\beta}_j$ is the correction for column j.

 N_{y} is the number of nodes in a column.

An approximate equation for $\overline{\alpha}$ is

$$B'_{i}\alpha_{i-1} + E'_{i}\alpha_{i} + H'_{i}\alpha_{i+1} = R'_{i}, i = 1, 2, \dots, N_{y} \quad (18)$$

in which

$$B'_{i} = -\sum_{j} B_{i,j};$$

$$E'_{i} = \sum_{j} (B_{i,j} + H_{i,j} + \frac{S_{i,j}}{\Delta t});$$

$$H'_{i} = -\sum_{j} H_{i,j};$$

$$R'_{i} = \sum_{i,j} R_{i,j}^{n};$$

$$R_{i,j}^{n} = B_{i,j} h_{i-1,j,k}^{n} + D_{i,j} h_{i,j-1k}^{n} + E_{i,j} h_{i,j,k}^{n}$$

$$+ F_{i,j} h_{i,j+1,k}^{n} + H_{i,j} h_{i+1,j,k}^{n} + \frac{S_{i,j}}{\Delta t} h_{i,j,k-1} - W_{i,j,k};$$
An approximate equation for $\overline{\beta}$ is

$$D'_{j}\hat{\beta}_{j-1} + E'_{j}\hat{\beta}_{j} + F'_{j}\hat{\beta}_{j+1} = R'_{j}, j = 1, 2, \dots, N_{x}$$
(19)

in which

$$D'_{j} = -\sum_{i} D_{i,j};$$

$$E'_{j} = \sum_{i} (D_{i,j} + F_{i,j} + \frac{S_{i,j}}{\Delta t});$$

$$F'_{j} = -\sum_{i} F_{i,j};$$

$$R'_{j} = \sum_{i} R_{i,j}^{n}$$

Equations 18 and 19 are derived with the following equations

$$\sum_{j=1}^{N_x} R_{j,j}^{n^*} = 0, \ i = 1, 2, \dots, N_y$$

and

$$\sum_{i=1}^{N_y} R_{i,j}^{n^*} = 0, \ j = 1, 2, \dots, N_x$$

which force the sum of residuals for each row and each column to zero when the vector \bar{h}^{n^*} is substituted into equation 8. Aziz and Settari (1972) give the exact equations for $\bar{\alpha}$ and $\bar{\beta}$ but point out that equations 18 and 19 are good approximations and, in practice, are easier to solve. For example, equation 19, which used alone is Watts' 1DC method, is written in matrix form as TECHNIQUES OF WATER-RESOURCES INVESTIGATIONS

$$\begin{bmatrix} E'_{1} & F'_{1} \\ D'_{2} & E'_{2} & F'_{2} \\ D'_{3} & E'_{3} \end{bmatrix} \begin{bmatrix} \hat{\beta}_{1} \\ \hat{\beta}_{2} \\ \hat{\beta}_{3} \end{bmatrix} = \begin{bmatrix} R'_{1} \\ R'_{2} \\ R'_{3} \end{bmatrix}$$

for the problem in figure 13. Equation 18 has an analogous form and both are easily solved by the Thomas algorithm.

Note that $\overline{\alpha}$ and $\overline{\beta}$ in the model are zero for those rows and columns in which one or more constant-head nodes are located. If $\overline{\alpha}$ and $\overline{\beta}$ were not zero it would not be possible to maintain a constant value at the appropriate nodes. As Watts (1973) points out, therefore, the procedure is most useful in simulations dominated by no-flow boundaries. For those simulations in which 2DC is useful, it is generally better to apply the corrections after several rather than after each LSOR iteration. After experimenting with a few problems, we have found it practical to apply 2DC after every 5 LSOR iterations.

LSOR acceleration parameter

The optimum value of ω for maximum rate of convergence lies between 1 and 2 and is commonly between 1.6 and 1.9. If only one or two runs will be made on a problem, it is probably best to choose an ω based on experience. If many runs will be made, it will be worthwhile to use an ω close to the optimum value. For simple problems ω_{opt} can be computed as explained, for example, by Remson, Hornberger, and Molz (1971, p. 188–199) using the equation

$$\omega = \frac{2}{1 + \sqrt{1 - \rho(G)}} \tag{20}$$

in which

$$\rho(G) \simeq \frac{\left| \xi_{\max}^{\dagger(n)} \right|}{\left| \xi_{\max}^{\dagger(n-1)} \right|}$$

 $\rho(G)$ is the spectral radius (dominant eigenvalue) of the Gauss-Seidel iteration matrix. For typical field problems it is possible to use equation 20 to estimate ω_{opt} in an iterative process if 2DC is not used. In the first simulation of the problem, set $\omega = 1.0$ and allow at least 100 iterations. In applying this method to problems 1, 2, and 3 it took 25 iterations to arrive at ω_{opt} for problem 2, but about 100 iterations to obtain ω_{opt} for problem 1 and 3. Obviously this method may involve a lot of computational effort to obtain ω_{opt} . More efficient methods using equation 20 have been devised to update ω during the iteration process. For example, Breitenbach, Thurnau, and van Poollen (1969) use a modified form of Varga's (1962) "power method," Carré's (1961) method is described by Remson, Hornberger, and Molz (1971, p. 199–203), and Cooley (1974) has a simple method for improving ω for transient problems.

Figure 14 illustrates the rate of convergence of LSOR and LSOR+2DC for test problems 1, 2, and 3 using different acceleration parameters chosen by trial and error. The values exceeding 100 iterations for problem 1 were estimated by using a plot, which is nearly a straight line, of the absolute value of the log of the maximum residual (defined by equation 28) versus the number of iterations. This plot was extrapolated to the value of maximum residual that corresponded roughly to the closure criterion chosen for the problem. The same procedure was used on problem 3 for values exceeding 200 iterations.

For problem 1 the optimum acceleration parameter is 1.87 for LSOR. Two-dimensional correction significantly improves the convergence rate of LSOR for this problem with an optimum acceleration parameter of 1.7. In problem 2, 2DC had no effect on the rate of convergence of LSOR because of the numerous constant-head nodes in the problem. Consequently, the optimum acceleration parameter is 1.6 with or without the application of 2DC. In problem 3, with LSOR oriented across the bedding, ω_{opt} is 1.88 for LSOR and about 1.70 for LSOR+2DC. Note in problems 1 and 3 that finding ω_{opt} for LSOR is more critical than with LSOR + 2DC. LSOR is poorly suited for problem 4 because too many nodes drop out in the iteration process if $1 < \omega < 2$. Satisfactory results for problem 4 at the expense of slow convergence are obtained if $\omega = 0.5$ (See fig. 23.)



FIGURE 14.—Number of iterations required for solution by LSOR and LSOR + 2DC using different acceleration parameters.

Alternating-direction implicit procedure

Peaceman and Rachford (1955) described the iterative, alternating-direction implicit procedure for solution of a steady-state (Laplace) equation in two space dimensions. This procedure, however, is equally applicable to transient problems where it has the advantage of allowing larger time steps than can be used with non-iterative ADI. (Non-iterative ADI was used by Pinder and Bredehoeft, 1968.) In the ADI technique, two sets of matrix equations are solved each iteration. The equations for rows in which head values along rows are computed implicitly and those along columns are obtained from the previous column computations are defined as

$$Dh_{j-1}^{n-\frac{1}{2}} + E_r h^{n-\frac{1}{2}} + Fh_{j+1}^{n-\frac{1}{2}}$$

= Q_r, j = 1,2,...,N_x (21a)

in which

$$E_r = -\left(D + F + \frac{S}{\Delta t} + M_i\right);$$

$$Q_{r} = -Bh_{i-1}^{n-1} + (B+H-M_{i})h^{n-1} - Hh_{i+1}^{n-1} - \frac{S}{\Delta t}h_{k-1} + W;$$

 M_i is the iteration parameter;

l is the iteration parameter index.

In matrix form equation 21a is

$$\bar{\bar{A}}_r \bar{h}^{n-\frac{1}{2}} = \bar{Q}_r.$$
 (21b)

To put equation 21b in residual form, add and subtract $\bar{A}_r \bar{h}^{n-1}$ to the right-hand side giving

$$\bar{A}_r \bar{h}^{n-\frac{1}{2}} = \bar{Q}_r - \bar{A}_r \bar{h}^{n-1} + \bar{A}_r \bar{h}^{n-1} \qquad (21c)$$

Rearrange equation 21c to read:

$$\overline{A}_r \overline{\xi}^{n-\frac{1}{2}} = \overline{R}_r^{n-1}$$
 (21d)

in which

$$\bar{\xi}^{n-\frac{1}{2}} = \overline{h}^{n-\frac{1}{2}} - \overline{h}^{n-1};$$

$$\bar{R}_r^{n-1} = \overline{Q}_r - \overline{A}_r \overline{h}^{n-1}.$$

Equation 21d is the ADI row formula in residual form. Its matrix form is the same as that for equation 17d and is solved for each row by the Thomas algorithm. To complete the first half of the ADI iteration, $\bar{h}^{n-\frac{1}{2}}$ is computed by

$$\bar{h}^{n-\frac{1}{2}} = \bar{h}^{n-1} + \bar{\xi}^{n-\frac{1}{2}}.$$

The equations in which head values along columns are considered implicitly and those along rows explicitly are written as:

$$Bh_{i-1}^{n} + E_{c}h^{n} + Hh_{i+1}^{n} = Q_{c}, i = 1, 2, \dots, N_{y}$$
 (22a)
in which

$$E_{c} = -(B + H + \frac{S}{\Delta t} + M_{l});$$

$$Q_{c} = -Dh_{j-1}^{n-\frac{1}{2}} + (D + F - M_{l})h^{n-\frac{1}{2}}$$

$$-Fh_{j+1}^{n-\frac{1}{2}} - \frac{S}{\Delta t}h_{k-1} + W.$$

Equation 22a in matrix form is

$$\overline{A}_c \overline{h}^n = \overline{Q}_c. \tag{22b}$$

By adding and subtracting $\overline{A}_c \overline{h}^{n-\frac{1}{2}}$ to the right-hand side of equation 22b, it can be put in the residual form

$$\overline{\bar{A}}_c \bar{\xi}^n = \overline{R}_c^{n-\frac{1}{2}}; \qquad (22c)$$

in which

.7

$$\begin{split} \bar{\xi}^{n} &= \overline{h}^{n} - \overline{h}^{n-\frac{1}{2}};\\ \overline{R}_{c}^{n-\frac{1}{2}} &= \overline{Q}_{c} - \overline{A}_{c} \overline{h}^{n-\frac{1}{2}} \end{split}$$

Equation 22c is solved for each column by the Thomas algorithm, and the vector \bar{h}^n for each row is obtained by the equation

$$\overline{h}^n = \overline{h}^{n-\frac{1}{2}} + \overline{\xi}^n.$$

A set of iteration parameters is computed by the equation

$$M_l = \omega_l \left(B + D + F + H \right)$$

in which ω ranges between a minimum defined by

$$\omega_{\min} = (\text{over grid}) \begin{bmatrix} \pi^{2} & 1 \\ 2\overline{N_{x}^{2}} & 1 \\ 1 + \left(\frac{T_{yy[i,j]}(\Delta x_{i})^{2}}{T_{xx[i,j]}(\Delta y_{i})^{2}}\right), \\ \frac{\pi^{2}}{2N_{y}^{2}} & \frac{1}{1 + \left(\frac{T_{xx[i,j]}(\Delta y_{i})^{2}}{T_{yy[i,j]}(\Delta x_{i})^{2}}\right)} \end{bmatrix}$$
(23a)

and a maximum given by

$$\omega_{\max} = \begin{cases} 1 & [T_{xx} \cong T_{yy}]; \\ 2 & [T_{xx} >> T_{yy} \text{ or } T_{yy} >> T_{xx}]. \end{cases}$$

The set of parameters are spaced in a geometric sequence given by

$$\omega_{l+1} = \gamma \omega_l \tag{23b}$$

in which

$$\ln \gamma = \frac{\ln \left(\omega_{\max} / \omega_{\min} \right)}{L - 1}.$$
 (23c)

L = the number of iteration parameters used.

The iteration parameters starting with ω_{\min} are cycled until convergence is achieved.

Equation 23a is based on a von Neuman error analysis of the normalized flow equations. (See, for example, Weinstein, Stone, and Kwan, 1969.) It will compute the optimum ω_{min} only for simple problems. For general problems ω_{\min} computed by equation 23a may or may not be close to the optimum ω_{\min} for the problem. This is illustrated in figure 15 in which the rate of reduction in the maximum residual for arbitrarily chosen minimum parameters is compared with that for ω_{min} computed with equation 23a. Ten parameters were used in problems 1 and 2, and four parameters were used in problem 3. The lines on figure 15 are meant to show the general trend only. The convergence rate using the best ω_{\min} in figure 15 is nearly the same as that computed with equation 23a for problem 1, but there is a significant difference in rates for problems 2 and 3. (See figs. 21 and 22.)

The other factor that may be critical in determining the rate of convergence using ADI is the number of parameters. In general, the number of parameters is chosen as 5 if ω_{max} $-\omega_{min}$ is about two orders of magnitude; if $\omega_{max}-\omega_{min}$ is three or more orders of magnitude, 7 or more parameters are chosen.

For the test problems, the number of iteration parameters were varied from 4 to 10 (fig. 16). The minimum parameter was calculated by equation 23a; the maximum parameter was 1 for problems 1 and 2 and was 2 for problem 3. The number of parameters had a relatively small effect in determining the rate of convergence for problems 1 and 3. For problem 2, however, the computations do not converge using 4 or 5 parameters. Problem 2 can be solved with ADI using 6 to 10 parameters with 10 parameters giving the most rapid convergence. ADI did not give satisfactory solutions for problem 4 (an excessive number of nodes always drop out of the solution) and, consequently, no results for problem 4 are shown in figure 16.

When difficulties occur with ADI in steadystate simulations, rather than experimenting with the critical minimum parameter or the number of parameters, it may be worthwhile to make the simulation a transient problem. In effect, $S/\Delta t$ is used as an additional iteration parameter. If the storage coefficient is not made too large or the time step too small,

 $X_{x}+1 \text{ elements}$ $X_{x}+1 \text{ elements}$ $V_{x}+1 \text{ elements}$ $D_{x}+1 \text{ elements}$

Direct solution of equation 24 by Gaussian elimination usually requires more work and computer storage than iterative methods for problems of practical size because \overline{A} decomposes into a lower triangular matrix with non-zero elements from B to E in each row and an upper triangular matrix with nonzero elements from E to H in each row. All of these intermediate coefficients must be computed during Gaussian elimination, and the coefficients in the upper triangular matrix must be saved for backward substitution.

To reduce the computation time and storage requirements of direct Gaussian elimination, Stone (1968) developed an iterative method using approximate factorization. In this approach a modifying matrix \overline{B} is added to \overline{A} forming $(\overline{A+B})$ so that equation 24 becomes

$$(\overline{\overline{A+B}})\,\overline{h} = \overline{Q} + \overline{\overline{B}}\,\overline{h}.$$
 (25)

 $(\overline{A+B})$ can be made close to \overline{A} but can be factored into the product of a lower triangular matrix \overline{L} and an upper triangular matrix \overline{U} , each of which has no more than three nonzero elements in each row, regardless of the size of N_x and N_y . Therefore, if the righthand side of equation 25 is known, simple steady state should be achieved within a reasonable number of time steps with rapid convergence at each time step.

Strongly implicit procedure

The set of equations (corresponding to equation 8) for the 3×3 problem in figure 13 may be expressed in matrix form as

Ā

$$\overline{h} = \overline{Q}$$
 (24)



recursion formulas can be derived, resulting in a considerable savings in computer time and storage. This leads to the iteration scheme

$$(\overline{\overline{A+B}})\,\overline{h}^n = \overline{Q} + \overline{\overline{B}}\,\overline{h}^{n-1}.$$
 (26)

In order to transform equation 26 into a residual form, $\overline{A}\overline{h}^{n-1}$ is subtracted from both sides giving

$$(\overline{\overline{A+B}})\,\bar{\xi}^n = \overline{R}^{n-1} \tag{27}$$

in which

$$\bar{\xi}^n = \bar{h}^n - \bar{h}^{n-1};$$

$$\bar{R}^{n-1} = \bar{Q} - \bar{A} \bar{h}^{n-1}.$$
(28)

The iterative scheme defined by equation 26 or 27 is closer to direct methods of solution (more implicit) than ADI (hence the term strongly implicit procedure or SIP). The SIP algorithm requires (1) relationships among the elements of \overline{L} , \overline{U} and $(\overline{A+B})$ defined by rules of matrix multiplication for the equation

$$\overline{\overline{L}} \ \overline{\overline{U}} = (\overline{\overline{A+B}}), \qquad (29)$$

and (2) relationships among the elements of $\overline{\overline{A}}$ and $(\overline{\overline{A+B}})$.

 \overline{L} and \overline{U} have the following form for a general 3×3 problem (much of the notation is adapted from Remson, Hornberger, and Molz, 1971);



FIGURE 15.—Reduction in the maximum residual for problems 1 to 3 for selected ω_{min} used to compute the ADI parameters.





FIGURE 16.—Number of iterations required for solution of the test problems with ADI using different numbers of parameters.

Because of the boundary conditions, the elements of $(\overline{A+B})$ inside squares will be zero for the 3×3 problem illustrated in figure 13. The relationships among the elements of \overline{L} , \overline{U} , and $(\overline{A+B})$ are

$$\alpha$$
 $=B$ (30a) $\alpha\delta_{i-1}$ $=\hat{C}$ (30b) β $=\hat{D}$ (30c) $\gamma + \alpha\eta_{i-1} + \beta\delta_{j-1} = \hat{E}$ (30d) $\gamma\delta$ $=\hat{F}$ (30e) $\beta\eta_{j-1}$ $=\hat{G}$ (30f) $\gamma\eta$ $=\hat{H}$ (30g)

where the *i* and *j* subscripts refer to the location on the model grid, not in matrix $(\overline{A+B})$.

In order to use equations 30a-30g as the basis of a numerical technique for solving equation 24 efficiently by elimination, relationships between the elements of \overline{A} and $(\overline{A+B})$ must be defined. One possibility is to let the elements correspond exactly and ignore the \hat{C} and \hat{G} diagonal in $(\overline{A+B})$. Stone (1968), however, found that this could not be used as the basis of a rapidly convergent iterative procedure. Instead, he defined a family of modified matrices starting with 30b and 30f.

Then the other elements of $(\overline{A+B})$ can be defined as equal to the corresponding elements in \overline{A} plus a linear combination of \hat{C} and \hat{G} . For example

$$\hat{B} = B + \phi_1 \hat{C} + \phi_2 \hat{G}$$

in which ϕ_1 and ϕ_2 are constants depending on the problem being solved.

What are appropriate linear combinations of \hat{C} and \hat{G} with the elements of \overline{A} ? If equation 27 is written for node (i,j), non-zero coefficients appear not only for the unknowns in the original difference equation but also for $\xi_{i-1,j+1}^n$ and $\xi_{i+1,j-1}^n$. This is illustrated in figure 17. To minimize the effects of the terms introduced in forming the modified matrix equation, $\overline{B}\overline{\xi}^n$ for the node (i,j) is defined as

$$C[\xi_{i-1,j+1}^{n} - \omega(\xi_{i-1}^{n} + \xi_{j+1}^{n} - \xi^{n})] + \hat{G}[\xi_{i+1,j-1}^{n} - \omega(\xi_{j-1}^{n} + \xi_{i+1}^{n} - \xi^{n})]$$
(31)

where the terms in parentheses are secondorder correct approximations for $\xi_{i-1,j+1}$, and $\xi_{i+1,j-1}$, respectively. (See Remson, Hornberger, and Molz, 1971, p. 226, for derivation of these approximations.) To consider these terms good approximations to $\xi_{i-1,j+1}$ and



FIGURE 17.-Coefficients of unknowns in equation 27.

 $\xi_{i+1,j-1}$ an iteration parameter, ω_i is added. The value of ω ranges between 0 and 1, and its computation is discussed at the end of this section.

With the definition of \overline{B} (31), the iteration scheme (equation 27) becomes

$$B\xi_{i-1}^{n} + D\xi_{j-1}^{n} + E\xi^{n} + F\xi_{j+1}^{n} + H\xi_{i+1}^{n} + \hat{C}[\xi_{i-1,j+1}^{n} - \omega(\xi_{i-1}^{n} + \xi_{j+1}^{n} - \xi^{n})] + \hat{G}[\xi_{i+1,j-1}^{n} - \omega(\xi_{j-1}^{n} + \xi_{i+1}^{n} - \xi^{n})] = R^{n-1}$$
(32)

Collecting coefficients in equation 32 associated with the nodal positions in the original difference equation gives the desired linear combinations of \hat{C} and \hat{G} with the elements of $\overline{\overline{A}}$ that define the remaining elements of $(\overline{\overline{A}+\overline{B}})$:

$$\hat{B} = B - \omega \hat{C} \tag{33a}$$

$$\hat{D} = D - \omega \hat{G}$$
 (33b)

$$\hat{E} = E + \omega \hat{C} + \omega \hat{G} \qquad (33c)$$

$$\hat{F} = F - \omega \hat{C}$$
 (33d)

$$\hat{H} = H - \omega \hat{G} \tag{33e}$$

The coefficient \hat{C} is obtained explicitly by combining equations 33a, 30a, and 30b as

$$\hat{C} = \frac{\delta_{i-1}B}{1+\omega\delta_{i-1}}.$$
 (34a)

Finally combining equation 33b and equations 30c and 30f gives

$$\hat{G} = \frac{\eta_{j-1}D}{1+\omega\eta_{j-1}}.$$
 (34b)

Equations 34, 33 and 30 (in that order) are the first part of the SIP algorithm.

Equation 28 written for node (i,j) is

$$R^{n-1} = Q - (Bh_{i-1}^{n-1} + Dh_{j-1}^{n-1} + Eh^{n-1} + Fh_{j+1}^{n-1} + Hh_{i+1}^{n-1}).$$

As in the Thomas algorithm, the vector $\bar{\xi}^n$ is obtained by a process of forward and backward substitution. Combining equations 27 and 29 gives

$$\overline{L}\overline{U}\overline{\xi}^{n} = \overline{R}^{n-1} \tag{35}$$

Define an intermediate vector \overline{V}^n by

$$\overline{U}\bar{\xi}^n = \overline{V}^n. \tag{36}$$

Then equation 35 becomes

$$\overline{L}\overline{V}^n = \overline{R}^{n-1}.$$
 (37)

 \overline{V}^n is first computed by forward substitution. This can be seen by writing equation 37 for node (i,j):

 $\alpha V_{i-1}^n + \beta V_{i-1}^n + \gamma V^n = R^{n-1}$

or

$$V^n = \left(R^{n-1} - \alpha V_{i-1}^n - \beta V_{j-1}^n\right) / \gamma.$$

The vector $\overline{\xi}^n$ may then be computed by backward substitution. Equation 36 for node (i,j) is

 $\xi^n + \delta \xi^n_{i+1} + \eta \xi^n_{i+1} = V^n$

or

$$\xi^n = V^n - \delta \xi^n_{i+1} - \eta \xi^n_{i+1}.$$

Stone (1968) recommends an alternating computational procedure. On odd iterations, the equations are ordered in a "normal" manner as shown in figure 13. On even iterations, the numbering scheme is changed to that illustrated in figure 18. This has the effect of making non-zero coefficients appear for the heads $h_{i-1,j-1}$ and $h_{i+1,j+1}$ (the X's in fig. 17) instead of $h_{i-1,j+1}$ and $h_{i+1,j-1}$ and significantly improves the convergence rate. Note that some of the recursion equations are modified by reordering the grid points in the "reverse" manner. The modifications required for the reverse algorithm are



FIGURE 18.—Reverse numbering scheme for 3× 3 problem.

$$\begin{split} \hat{C} &= \frac{\delta_{i+1}H}{1+\omega\delta_{i+1}}; \\ \hat{B} &= H - \omega \hat{C}; \\ \hat{H} &= B - \omega \hat{G}; \\ \gamma &= E - \alpha \eta_{i+1} - \beta \delta_{j-1}; \\ V^n &= (R^{n-1} - \alpha V_{i+1}^n - \beta V_{j-1}^n) / \gamma; \\ \xi^n &= V^n - \delta \xi_{j+1}^n - \eta \xi_{i-1}^n. \end{split}$$

The iteration parameters are computed by equations given in Stone (1968). For variable transmissivity and grid spacing, Stone's equation is

$$(1 - \omega_{\max}) = \sum_{i=1}^{N_{y}} \sum_{j=1}^{N_{x}} \operatorname{Min} \left[\frac{2(\delta x_{j})^{2}}{1 + \left(\frac{T_{yy[i,j]}(\delta x_{j})^{2}}{T_{xx[i,j]}(\delta y_{i})^{2}}\right)} + \frac{2(\delta y_{i})^{2}}{1 + \left(\frac{T_{xx[i,j]}(\delta y_{i})^{2}}{T_{yy[i,j]}(\delta x_{j})^{2}}\right)} \right] \div (N_{x} \times N_{y}) \quad (38)$$

in which

$$\delta x = \Delta x_j / \text{width of model}$$

 $\delta y = \Delta y_i / \text{length of model}$

Equation 38 computes an arithmetic average of ω_{max} for the algorithm.

The remaining iteration parameters are computed by

$$1 - \omega_{l+1} = (1 - \omega_{\max})^{l/(L-1)}, l = 0, 1, \dots, L-1$$

in which L is the number of parameters in a cycle.

Stone (1968) recommends using a minimum of four parameters, each used twice in succession, starting with the largest first. Weinstein, Stone, and Kwan (1969), however, indicate that it is not necessary to start with the largest parameter first or to repeat them.

The results using different numbers and sequences of parameters for the three test problems are shown in figure 19. Except for the sequence 4, 3, 2, 1 in problem 1 the number of iterations required for solution varies up to a maximum of 50 percent for the parameter sequences tested. Several parameter sequences (for example, 1, 2, 3, 4, 5) give convergence near the maximum observed rate for all problems. This result suggests that conducting numerical experiments to determine the best sequence of parameters for a particular problem is generally not justified.

Weinstein, Stone, and Kwan (1969) have a slightly different definition of the maximum parameter $(1-\omega_{max}=ADI minimum parame-$ ter). Their definition of the maximum parameter (which is the maximum over the model, not the arithmetic average of values computed for each node) was used in solving several test problems. In every case convergence was faster using equation 38 to compute the maximum parameter.

Stone (1968) states that a more general form of equation 27 includes another iteration parameter, β' , to multiply the term \overline{R}^{n-1} . His experience indicated, however, that values of β' other than unity did not generally improve the method. In contrast, the use of β' other than unity has proven to be effective for some of the test problems. In fact, for the fourth problem, a value of β' less than unity is required to obtain a reasonable solution using SIP. Results for problem 4 are not shown in figure 19 because the best sequence of parameters (No. 3) for problem 2 was used in experimenting with the parameter β' .



FIGURE 19.—Iterations required for solution of the test problems by SIP using different numbers and sequences of parameters.

Comparison of Numerical Results

The rate of convergence using different numerical techniques for solving the test problems is compared in figures 20 to 23. The best results from the experiments with each iterative technique are used in the comparisons. Two curves (except for fig. 23) are shown for SIP: one with the parameter $\beta'=1$ and the other with the best rate of convergence for $\beta' \neq 1$. The sequence of ω parameters is the same for both curves. Two curves are also shown for ADI: one in which the minimum parameter was calculated with equation 23a (indicated by an asterisk in the figures); the other with the best minimum parameter shown on figure 15.

In figures 20 to 23 the absolute value of the maximum residual for each iteration is plotted versus computation time where one unit of work is equal to the time required to complete one SIP iteration. Relative work per iteration is about 1 for ADI, 0.6 for LSOR. and 0.8 for LSOR+2DC. The maximum residual for SIP and ADI fluctuates from a maximum to a minimum over each cycle of parameters. For clarity, the curves connect the local minima for these two methods. Comparisons in figures 20-23 should be made on the basis of the horizontal displacement of the curves, not on the basis of the termination of the curves. This is similar to the type of comparisons made by Stone (1968).

Figure 20 shows the results for problem 1 (10 parameters for ADI, $\omega = 1.87$ for LSOR, $\omega = 1.7$ for LSOR+2DC, parameter sequence, 1,1,3,3,5,5,2,2,4,4,6,6, for SIP). Of the sequence of β' parameters tried, the minimum work required to reduce the residual is obtained with $\beta' = 1.4$, but this is only moderately better than using $\beta' = 1.0$. ADI converges as rapidly as SIP for the first cycles of iteration, but from that point on converges slower than the other iterative techniques. The two ADI curves show about the same rate of convergence for this problem. Next to SIP, LSOR+2DC is most attractive for this problem.



FIGURE 20.—Computational work required by different iterative techniques for problem 1.

The results for problem 2 are shown in figure 21 (10 parameters for ADI, $\omega = 1.6$ for LSOR and LSOR+2DC, parameter sequence 1,2,3,4,5 for SIP). SIP requires the least amount of work for this problem (using $\beta' \neq 1.0$ does not significantly reduce the work required). LSOR and ADI using the best ω_{\min} from figure 15 are competitive with SIP. ADI using ω_{\min} computed with equation 23a requires about twice as much computational work. LSOR and LSOR+2DC take the same number of LSOR iterations so that the extra work required for 2DC is wasted for this problem.

In figure 22, the results using 4 parameters for ADI, the parameter sequence 1,2,3,4 for SIP, $\omega = 1.88$ for LSOR and $\omega = 1.70$ for LSOR+2DC are plotted for problem 3. In this problem LSOR (with solution lines oriented along columns), ADI with ω_{min} computed with equation 23a, and SIP with $\beta' = 1$ are competitive. Convergence is significantly improved by adding 2DC to LSOR, choosing the best ω_{min} from figure 15 for ADI and letting $\beta' = 1.5$ with SIP.



iteration for LSOR+2DC. Konikow (oral

FIGURE 24.—Number of iterations required for solution of problem 4 by SIP using different values of β' .

commun., 1975) was unable to obtain a solution to problem 4 using ADI due to oscillations that eliminated nodes that should have been in the solution. This problem occurred not only with ADI but also with LSOR and LSOR+2DC with $\omega > 0.6$ and with SIP with $\beta' > 0.6$. The oscillations are apparently caused in part by the nonlinearities of the water-table problem and the necessity to calculate transmissivity at the known iteration level. In a water-table simulation the transmissivity is set to zero and nodes are dropped from the aquifer if the computed head is below the base of the aquifer. For problem 4, at least 3 nodes should be dropped with the initial conditions used.

A solution to problem 4 in which 3 to 4 nodes are dropped is obtained with LSOR and LSOR+2DC when $\omega = 0.5$ at the expense of slow convergence. Clearly the most suitable method for this problem is SIP with $\beta' \leq 0.6$ (fig. 23). In effect the use of $\beta' < 1$ for SIP and $\omega < 1$ for LSOR represents "underrelaxation" and has the effect of dampening oscillations of head from one iteration to the next. This reduces the tendency for incorrect deletion of nodes from the solution.

Solution of problem 4 emphasizes the advantage of the extra SIP iteration parameter. The optimum value of β' inferred from figure 24 is about 0.5. Note in figure 24 that an additional node is dropped for $\beta'=0.5$ and 0.6. However, the effect of this node on the remainder of the solution is negligible. For $\beta'>0.6$, either convergence was not obtained or excessive numbers of nodes were dropped for those cases that did converge.

The numerical experiments included in this report support the general conclusions of Stone (1968) and Weinstein, Stone, and Kwan (1969) that SIP is a more powerful iterative technique than ADI for most problems. SIP is attractive, not only because of its relatively high convergence rates but because it is generally not necessary to conduct numerical experiments to select a suitable sequence of parameters. SIP has the disadvantage of requiring 3 additional $N_x \times N_y$ arrays.

For the first three problems examined here, ADI is a slightly better technique than LSOR when ω_{min} near the optimum is used. Although this result agrees with Bjordammen and Coats (1969) who concluded that ADI is superior to LSOR for the oil reservoir problems they investigated, it is deceptive because less work is required to obtain ω_{opt} for LSOR than is required to find the best ω_{min} for ADI by trial and error. Furthermore, LSOR is clearly superior to ADI in application to problem 4 where a solution was not possible with ADI as used in this simulator.

LSOR + 2DC seems to be particularly useful with problems dominated by no-flux boundaries. The correction procedure can significantly improve the rate of convergence of LSOR even in problems such as problem 3 where all β_j are zero and non-zero α_i occur for the lower half of the model only.

Considerations in Designing an Aquifer Model

Boundary conditions

An aquifer system is usually larger than the project area. Nevertheless the physical boundaries of the aquifer should be included in the model if it is feasible. Where it is impractical to include one or more physical boundaries (for example, in an alluvial valley that may be several hundred miles long) the finite-difference grid can be expanded and the boundaries located far enough from the project area so that they will have negligible effect in the area of interest during the simulation period. The influence of an artificial boundary can be checked by comparing the results of two simulation runs using different artificial boundary conditions.

Boundaries that can be treated by the model are of two types: constant head and constant flux. Constant-head boundaries are specified by assigning a negative storage coefficient to the nodes that define the constanthead boundary. This indicates to the program that these nodes are to be skipped in the computations. A constant flux may be zero (impermeable boundaries) or have a finite value. A zeroflux boundary is treated by assigning a value of zero transmissivity to nodes outside the boundary. The harmonic mean of the transmissivity at the cell boundary is zero, and consequently, the flux across the boundary is zero. A no-flow boundary is inserted around the border of the model as a computational expediency, and constant-head or finite-flux boundaries are placed inside this border. A finite-flux boundary is treated by assigning recharge (or discharge) wells to the appropriate nodes. Figure 25 illustrates various types of boundary conditions.

The type of boundaries appropriate to the field problem may require careful consideration. In particular, should streams be treated as constant-head boundaries or are they more realistically treated as partially penetrating with a leaky streambed? If a leaky streambed is used, note that the leakage occurs over the area of the blocks assigned to the stream. If the area of the streambed is less than the area of the blocks, the ratio of streambed hydraulic conductivity to thickness can be proportionately reduced to make the amount of leakage realistic.

Initial conditions

In many simulations, the important results are not the computed head but the changes in head caused by a stress such as pumping wells. For this objective in a confined aquifer for which the equations are linear, there is no need to impose the natural flow system as the initial condition since the computed drawdown can be superimposed on the natural flow system, if desired.

If initial conditions are specified so that transient flow is occurring in the system at the start of the simulation, it should be recognized that water levels will change during the simulation, not only in response to the new pumping stress, but also due to the initial conditions. This may or may not be the intent of the user.

To start from steady-state conditions in which flow is occurring, the model can be used to compute the initial head by leaving out the new stress (for example, wells) and setting all storage terms to zero. This is also a useful calibration procedure to compute unknown terms such as the ratio of hydraulic conductivity to thickness for leakage.

Designing the finite-difference grid

In designing a finite-difference grid, the following considerations should be kept in mind:

- 1. Nodes representing pumping and observation wells should be close to their respective positions to facilitate calibration. If several pumping wells are close together, their discharge may be lumped and assigned to one node since discharge is distributed over the area of the cell.
- 2. Boundaries within the project area should be located accurately. Distant boundaries can be located approximately and with fewer nodes by expanding the grid. In expanding a finite-difference grid in the positive X direction, experience has shown that restricting the ratio $\Delta X_j / \Delta X_{j-1} \leq 1.5$ will avoid large truncation errors and possible convergence problems.
- 3. Nodes should be placed close together in areas where there are spatial changes in transmissivity. For example, in cross-sectional problems with aquifers separated by confining beds, many layers of nodes are required in the confining bed to obtain a good approximation of the head distribution (and consequently the flux) during transient conditions.
- 4. The grid should be oriented so that a minimum of nodes are outside the aquifer. The orientation of the grid with respect to latitude and longitude or some other geographic grid system would be a secondary consideration. However, if the aquifer is anisotropic, the grid should be oriented with its axes parallel to the principal directions of the transmissivity tensor. Otherwise,



EXPLANATION

Node symbols

Inside aquifer (transmissivity >0)

- w Discharge well
- R Recharge well
- ♥ Constant head
- Node without wells or specified head

Outside aquifer

O Transmissivity = 0

Aquifer boundary

Mathematical boundary

DIML Number of rows

DIMW Number of columns

Boundary conditions



Constant head

Constant flux

$$\boxed{\begin{array}{c} \frac{\partial h}{\partial x} = 0 \\ R & \frac{\partial h}{\partial x} = C \end{array}}$$

FIGURE 25.---Variable, block-centered grid with mixed boundary conditions.
the flow equation would include crossproduct terms and the solution would be restricted to ADI and LSOR because additional diagonals appear in the coefficient matrix and SIP, in its usual form, cannot be used.

- 5. The rows should be numbered in the short dimension for the alphameric plot on the line printer or for plotting data with an X-Y plotter. On these plots, the X-direction is vertical and, for practical purposes, this dimension is unlimited. The Y direction is across the page which limits this dimension to the maximum width of the page. (See fig. 26.)
- 6. The core requirements and computation time are proportional to the number of nodes representing the aquifer.

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COMPUTER PROGRAM AND RELATED DATA

Attachment I

Att	achment I	g` q = \$	transient part of $q'(Lt^{-1})$; evapotranspiration flux (Lt^{-1}) ;
	Notation	gro Q	known term in difference equa-
$\overline{\overline{A}}$	coefficient matrix; coefficient matrices for ADI col-	Qos	tion; maximum evapotranspiration rate (Lt^{-1}) ;
\bar{A}_{λ}	umn and row equations; LSOR coefficient matrix;	Q., Q.	known terms in equations defin- ing ADI;
b	saturated thickness of the $acuifer (L)$:	Q	well discharge $(L^{3}t^{-1})$;
\overline{B} B, D, E, F, H	modifying matrix for SIP; coefficients in difference equa-	r	LSOR; radial distance from center of
	tion;	-	pumping well (L) ;
B, C, D, E, F, G, H B', D', E', F', H'	coefficients of equations defining	r.	(L);
DT A T	2DC;	Γ	well radius (L);
<i>BE</i> , <i>G</i> , <i>W</i>	notation used in Thomas algorithm;	r_1	radius equivalent to the average grid spacing for the well block
Er, Eo	coefficients in equations defining	7	(L);
EM	ADI;	R Dn*	residual;
<i>L I</i> :	depth below land surface at	R' P'	sum of residuals for 2DC;
	ceases (L) :	R_{c}, R_{r}	residuals for ADI column and
G	elevation of the land surface		row computations;
	(L);	R_{λ}	LSOR residual;
h	hydraulic head (L) ;	S	storage coefficient (dimension-
ስ፣ አመ	intermediate head value (L);	a	less);
	(L);	~	fining bed (L^{-1}) ;
hι,j,o 2	initial head in the aquifer (L) ;	S_{ν}	specific yield (dimensionless);
<i>Ri</i> , <i>j</i> , <i>o</i>	hydraulic head on the other side of the confining bed (L) ;	t 	period (t);
h _w	hydraulic head in a well (L) ;		transmissivity $(L^{*}t^{-1})$;
Η , , ,	saturated thickness of the aquifer at radius $r_e(L)$;	T_L	transient leakage coefficient $(t^{-1});$
Hw	saturated thickness of the aquifer at radius $r_{v}(L)$;	$T_{xx}, T_{xy}, T_{yx}, T_{yy}$	components of the transmissivity tensor (L^2t^{-1}) ;
i	index in the y dimension;	\overline{U}	upp <u>er tri</u> angular factor of
j	index in the x direction;		$(\overline{A+B});$
k	time index;	V	intermediate vector in SIP al-
K 22, K yy	principal components of the hydraulic conductivity tensor	W(x,y,t)	gorithm; volume flux per unit area (Lt ⁻¹);
TZ 1	$(Lt^{-1});$	a	row correction for LSOR;
K.	hydraulic conductivity of the confining bed (L/t) ;	α, β, γ, δ, η β	elements of factors of $(A+B)$; parameter in Hantush (1960)
1	iteration parameter index;	â	solution;
L	number of iteration parameters	β β'	column correction for LSOR;
\overline{L}	In a cycle; lower triangular factor of	ρ γ	constant used in calculating ADI
5	$(\overline{A+B});$		parameters;
m	thickness of the confining bed (L);	$\delta_x, \delta_y \\ \Delta h$	normalized grid spacing; head change between adjacent
М	vector of ADI parameters;		nodes (L) ;
n	iteration index;	Δt	time increment (t) ;
Ν,	number of arrays required for the options;	Δx	space increment in the x direc- tion (L);
Ν.	number of nodes in a row;	Δy	space increment in the y direc-
N _v	number of nodes in a column;		tion (L) ;
q'	flux from a confining bed (Lt^{-1}) ;	3	closure criterion (L) ;

.

.

3	vector of change in head over
	an iteration;
₽(G)	spectral radius of Gauss-Seidel
	iteration matrix;
ϕ_1, ϕ_2	constants in definition of co-
	efficients of $(\overline{A+B})$;
ω	acceleration parameter;
ω_l	iteration parameter;
ω_{\max}	maximum iteration parameter;
ω_{min}	minimum iteration parameter;
wopt	optimum acceleration parameter.

Attachment II, Computer Program

Main program

The first function of the main program is to dimension the arrays for the field problem being simulated. The algorithm allocates storage space reserved in a vector, Y. Some arrays are required for every simulation; others are needed only if certain options are specified. The information needed to allocate space to the arrays is contained in the Group I data cards which are read by the main program (see Attachment III).

Once the model is compiled, it does not need to be recompiled for a new field problem unless (1) the logic is changed or (2) the vector Y is not dimensioned large enough for the new problem. The minimum dimension of the vector Y (YDIM) can be computed by

$$YDIM \simeq (15 + N_a) N_a N_y \tag{39}$$

in which N_a is the total number of arrays required for the options (from table 2).

Equation 39 is approximate, but normally will give a value that is sufficient for the simulation. The exact dimension required is

	Table	2Number	of	arrays	required	for	the	options
--	-------	---------	----	--------	----------	-----	-----	---------

Option	Number of arrays
Water Table	3
Conversion ¹	1
Leakage	3
Evapotranspiration	1
SIP	4

¹Conversion also requires the arrays for the water table option.

printed on the first page of the output as 'WORDS OF VECTOR Y USED=XXXX'.

In the second part of the main program, the location of the initial addresses of the arrays are passed to the subroutines. (See table 3 for details.) The variables in table 3 defining the dimensions of the arrays are defined in Attachment VI; the first four arrays and XII are double precision.

The last part of the main program controls the sequence of computations illustrated by the generalized flow chart (Appendix V). In the flow chart, the routines are lettered in sequence starting with the main program. Entry points for the routines are numbered in sequence along the left side of the chart. Exits from a routine are indicated by circles containing the entry point of the routine to which control passes. A break occurs in the flow chart following an unconditional exit. Variables used in the flow chart are defined in Attachment VI.

Subroutine DATAI

Instructions for the preparation of the data deck are given in Attachment III. Data may be input to the model in any consistent set of units in which second is the time unit. It is organized into four groups: Data in groups I and II are the simulation options and scalar parameters: group III cards are used to initialize the arrays. These three groups are required for each new simulation. Group IV contains data that varies with each new pumping period. The program permits changing well discharge and the time parameters each pumping period, but the program can be modified to read other data (for example, recharge rate) with this set of cards.

Time parameters

The time parameters include the initial time step, DELT; a multiplication factor for increasing the size of the time step, CDLT; the number of time steps, NUMT; and the simulation period, TMAX. Since the rate of water-level decline decreases during a pumping period, the time step is increased by the factor CDLT each step (commonly 1.5). For

Table 3.—Arrays passed	I to th	e subroutines	and their	relative	location	in the	vector \
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A	Sequence	Subroutine						
Аггау	vector Y	DATAI	STEP	SOLVEI	COEF	CHECKI	PRNTAI	Dimensions
РНІ	1	×	×	×	×	×	×	(IZ, JZ) *8
BE G TEMP	2 3 4	 	 	× × ×			 	IMAX *8 IMAX *8 IMAX *8
KEEP	5		×	×	×	×		IZ, JZ
STRTSURI	7	××	×	Ŷ	Ŷ	x	 	IZ, JZ
T TR	9 10	Ŷ	Ŷ	×	Ŷ	××		IZ, JZ
TCS	11 12	×		×	××	××	×	IZ, JZ IZ, JZ
QRE WELL	13 14 15	××	$\overline{\times}$	××	×	××	×	IZ, JZ IZ, JZ
SL	16	Ŷ		Ŷ	Ŷ	<u></u>		IZ, JZ IZ, JZ
PERM BOTTOM SY	17 18 19	×××	× ×		×××	× × ×		IP, JP IP, JP IP, JP
RATE RIVER M	20 21 22	× × ×			×××	× × ×		IR, JR IR, JR IR, JR
ТОР	23	×	×		×	×		IC, JC
GRND	24	×			×	×		IL, JL
DEL ETA V XI	25 26 27 28	 	 	××× ××		 	 	IS, JS IS, JS IS, JS IS, JS IS, JS
DELX DDN BETA	29 30 31	× 	× × 	× ×	× 	× 	× 	JZ JZ JZ
DELY ALFA	32 33	× 	× 	××	× 	× 	× 	IZ IZ
WR NWR	34 35	××	××		 		 	IH IH, 2
XII	36			×				IMAX *8
TEST 3	37		×	×				IMX1

any time step (k) the time increment is given by

$\text{DELT}_k = \text{CDLT} * \text{DELT}_{k-1}$.

 $DELT_o$ is the time step recorded on the data card.

The program has two options for selecting the time parameters:

 To simulate a given period of time, select CDLT and an appropriate DELT_o, and set NUMT greater than the expected number of time steps. The program computes the required initial DELT_o (which will not exceed the value of DELT_o coded on card 1 of group IV) and NUMT to arrive exactly at TMAX on the final time step. In a simulation of one pumping period in which results are required at several specific times, the simulation can be broken into several "pumping periods." Each period will have the same pumpage, and TMAX is used to specify the appropriate times for display of results.

2. To simulate a given number of the time steps, set TMAX greater than the expected simulation period and the program will use DELT_o, CDLT, and NUMT as specified on the time parameter card.

To minimize the error due to approximation of the time derivative, several time steps should be simulated before the first step at which results are displayed. This suggestion should be followed unless the system is nearly steady-state before the results are needed. In this case a one-step simulation may be satisfactory, but this approach should be checked by making one run as a multistep simulation so that the results can be compared.

For steady-state simulations, set the storage coefficient and (or) specific yield of the aquifer and the specific storage of the confining bed to zero. Compute for one time step of any length (for example, set TMAX = 1, NUMT=1, CDLT=1, DELT=24) and the program should iterate to a solution. The maximum permitted number of iterations (ITMAX) should be larger for steady-state than for transient simulations. If the calculations do not converge to a solution within a reasonable number of iterations, it may be necessary to use a transient simulation for enough steps to attain steady state (see also the discussion of ADI iteration parameters) or use another numerical technique.

Initialization

In addition to reading data and computing the time parameter, this routine initializes other arrays and scalar parameters. In particular, note that the leakage coefficient, TL, will equal $K'_{i,j}/m_{i,j}$ and can be computed once for the entire simulation if the specific storage of the confining bed is zero. The computation of the steady leakage term, SL, and the division of well discharge by the area of the cell need to be done only once for each pumping period. At the beginning of each pumping period the starting head (STRT) and the simulation time (SUMP) used in computing transient leakage are initialized.

Subroutine STEP

Subroutine STEP initializes variables for a new time step, checks for steady-state conditions after a solution is obtained for the time step, and controls the printing and punching of results and the writing of results on disk. If head values are punched at the end of the simulation or are written on disk, they can be used to extend the simulation or as input to plotting routines. (See the program by Cosner and Horwich, 1974.) Currently, a general program is being written to display results in various forms on the line printer and plotters; it is described in detail in another section of this report.

In the check for steady state during transient simulations, the head change over a time step is computed. If the absolute value of change at all nodes is less than EROR, the message 'STEADY STATE AT TIME STEP X' is printed. The program then prints all desired output for the final time step (X) and proceeds to read data for the next pumping period, if any.

Maximum head change for each iteration

The printed results are explained in the section on theory and in the discussions of subroutines COEF, CHECKI, and PRNTAI or are self explanatory, except for the listing of the absolute value of the maximum head change for each iteration. This information is useful if convergence is slow with ADI or SIP because it may indicate that a slightly larger error criterion will give a satisfactory solution with considerably fewer iterations.

Subroutine SOLVE

The three SOLVE routines, SOLVE1, SOLVE2, and SOLVE3 are, respectively, SIP, LSOR and ADI. They have been described in previous sections, but a few additional comments are necessary.

In these routines and in subroutine COEF, the usual (I,J) notation has been replaced in favor of single-subscript notation. Less time is involved in finding the value of a variable with a single subscript than in finding the value of one with a double subscript and, as a consequence, computational efficiency is improved. The five variables used as subscripts in this notation are defined in Attachment VI.

SIP iteration parameters

The algorithm in ITER1 permits computation of the iteration parameters in increasing or decreasing order and repeat of parameters depending on the initialization of the vector IORDER. Note that LENGTH is twice the number of different parameters and that the DATA statement that initializes IORDER assumes LENGTH=10. Replace the DATA statement with a READ statement if additional flexibility is desired in choosing the order of parameters without recompiling the subroutine.

Exceeding permitted iterations

If the permitted number of iterations for a time step is exceeded, the message 'EX-CEEDED PERMITTED NUMBER OF ITERATIONS' is printed. Following the message the mass balance, head matrix, etc., as specified in the options are printed for the final iteration. This information is useful in determining the cause of the nonconvergence. Before terminating the run, the mass balance and head values will be punched if PUNC was specified in the options or written on disk if IDK2 was specified. With punched output or results on disk, the user has the option to extend the number of iterations if it appears that a solution can be obtained. If iterations are exceeded on the first time step, the head values saved (punched or written on disk) were computed in the last iteration. If iterations are exceeded on a subsequent time step, KT, the head values and mass-balance parameters saved are the results for time step KT-1.

Subroutine COEF

Most of the calculations for coefficients used in the solution of the numerical schemes are done in this routine. The more extensive computations except those described in the section on theory are discussed in the following paragraphs.

Transient leakage coefficients

The algorithm for the transient parts of equations 9 and 10 is the same except for two conditional statements that recompute PPT and DENOM if dimensionless time is in the range for applying equation 9. In performing the infinite summation, the code checks for the significance of additional terms, but in any case limits the summation to a maximum of 200 terms. The minimum and maximum values of dimensionless time, TMIN and TT, are retained and printed with the results for the time step so that the user will know whether or not transient leakage effects are significant.

Transmissivity as a function of head

The transmissivity for water-table or combined water-table-artesian aquifers is computed as a function of the saturated thickness of the aquifer. If a cell (except a cell with well discharge) goes dry, a message 'NODE I, J GOES DRY' is printed, the transmissivity for the cell is set to zero, and the head is set to the initial surface (so that the location of the cell will show up in the output). No provision is made to permit the cell to resaturate in subsequent pumping periods because the additional code necessary to accommodate this special situation is not warranted in a general program.

When a cell with well discharge goes dry (that is, a hypothetical well with radius r_e goes dry), the program terminates the computation with printed output, and, if specified in the options, saves the results. Printed output is headed by 'WELL I, J GOES DRY' followed by drawdown when the well went dry. If results for the previous time step were not printed, drawdown and a mass balance (if specified in the program options) for the previous time step are printed. Finally, if specified in the options, mass-balance parameters and head values for the previous time step are punched or written on disk so that the user has the option of continuing the simulation after modifying the well discharge.

TR and TC coefficients

The TR and TC arrays save values that are used repeatedly in the algorithm. They are computed once for artesian problems and each iteration for water-table and combined artesian-water-table simulations. TR (I,J) is the harmonic mean of $T_{xx}(I,J)/DELX(J)$, $T_{xx}(I,J+1)/DELX(J+1)$; TC (I,J) is the harmonic mean of $T_{yy}(I,J)/DELY(I)$, T_{xx} (I+1,J)/DELY(I+1).

Subroutine CHECKI

A mass balance is computed in this routine. The results are expressed in two ways: (1) as a cumulative volume of water from each source and each type of discharge and (2) as rates for the current time step.

In the cumulative mass balance, storage is treated as a source of water. Flow to and from constant-head boundaries is computed with Darcy's law using the gradients from constant-head nodes to adjacent nodes inside the aquifer. Other computations in the algorithm are self explanatory.

The difference between the sum of sources and sum of discharges from the system is usually less than 1 percent. A larger error, however, does not necessarily mean that the results are poor; it may be due to lack of precision in calculating the mass balance. This has been observed, for example, if a leaky streambed is given a large K'/m ratio so that it is effectively a constant-head boundary. The leakage computation is inaccurate if the head values at a stream node are identical to 6 or 7 significant figures and they are stored as single precision variables.

To the right of the cumulative mass balance are printed the flow rates for the current time step. They are self explanatory except for leakage. "Leakage from previous pumping period" is the leakage resulting from gradients across the confining bed at the start of the current pumping period. The "total" leakage is the sum of leakage due to the initial gradients plus leakage induced by head changes during the current pumping period.

Subroutine PRNTAI

This routine prints a map of drawdown and hydraulic head. Up to three characters are plotted for each cell with the rightmost character as close to the location of the node as the printer will allow. An option to permit the printing of results at different scales in the x and y dimensions is useful for cross sections. This routine is useful for displaying results during calibration runs. More elegant graphical displays for final results are described in another section.

The user specifies XSCALE and YSCALE, the multiplication factors required to change from units used in the model to units used on the map: DINCH, the number of map units per inch; FACT1 and FACT2, the multiplication factors for adjusting the values of drawdown and head to be plotted, respectively; and MESUR, the name of the unit used on the map. As an example, assume that the length unit used in the model is feet, the map is to be scaled at 3 miles per inch and drawdown values at 1 foot increments and head values at 10 foot increments are to be plotted. Then XSCALE = YSCALE = 5280, DINCH =3, FACT1=1, FACT2=0.1; and MESUR = MILES.

To print a map of maximum possible size, number the rows in the short dimension to take advantage of the orientation of the map on the computer page where the X direction is vertical and the Y direction horizontal. (See fig. 26.) The origin is the upper lefthand corner of the block for row 2, column 2. Orienting the map with the origin in the upper left-hand corner, the right and bottom sides of the map include the node locations for the second to last column and row, respectively. The border is located to the nearest inch outside these node locations and may or may not fall on the cell boundaries depending on the scaling. The map is automatically centered on the page and is limited to a maximum of 12 inches (300 mm) in the Y direction. If the parameters for a map are specified such that the Y dimension is more than 12 inches (300 mm) adjustments are automatically made to fit the map within this limit. A common mistake is to specify a value for Y scale that is less than 1.0. This generates the message 'NOTE: GENERALLY SCALE SHOULD BE>OR = 1.0,' and a suit-



FIGURE 26.—Orientation of map on computer page.

able adjustment is made to DINCH. In the X direction, the map is limited only by the dimension of the NX vector. (For example, when the dimension of NX is 100, the map is limited in the X direction to 100-1=99 inches (2500 mm).) Several parameters (PRNT, BLANK, N1, N2, N3, and XN1) are initialized in the BLOCK DATA routine to values that assume the line printer prints 6 lines per inch, 10 characters per inch, and 132 characters per line. These parameter values may need to be changed for a line printer with other specifications.

The PRNTAI subroutine can be modified to cycle a set of alphameric symbols for drawdown. If this type of map is desired, remove the C from column 1 of statements PRN1060 and PRN1230. This will cycle the symbols 1,2,3,4,5,6,7,8,9,0 for drawdown. To plot a different set of symbols will require modification of the initialization of SYM in BLOCK DATA. To cycle more than 10 symbols will require more extensive changes to the initialization of SYM and modifications to the code in ENTRY PRNTA.

BLOCK DATA routine

The BLOCK DATA routine initializes scalar parameters and arrays used in PRNTAI and other subroutines. The unit numbers for card reader, line printer, and card punch are commonly 5, 6 and 7, respectively. At computer installations where other numbers are used, change the initialization of P, R, and PU.

Technical information

Storage requirements

Using the FORTRAN G, Level 21 compiler, the source code and fixed-dimension arrays require 100K bytes of memory (88K bytes if only one SOLVE routine is complied). The storage requirements including all options but not including storage requirements for reading and writing on disk are (100+X/256) K bytes where X is the dimension of the vector Y in the main program. Subtract 14K bytes from the values if the FORTRAN H, OPT=2 compiler is used. The FORTRAN G compile step requires 120K bytes of memory and the FORTRAN H, OPT=2 compiler requires 218K bytes of memory.

Computation time

Computation time is a function of so many variables that no general rule can be stated. For example, the simulation of a nonlinear water-table problem requires many more computations per time step than does the simulation of a linear artesian-aquifer problem.

As an example, the simulation of a linear aquifer system (problem 2) with a grid of 25×38 required 45 seconds for 40 iterations with the program compiled under FORTRAN G. This is about 0.002 seconds for each node inside the aquifer each iteration on the IBM 370/155. A significant reduction (about 1/3)

in execution time can be achieved by using the FORTRAN H compiler which generates a more efficient code than the FORTRAN G compiler.

Further significant reductions in execution time can be achieved if the model is designed for a specific problem. Problem 3, for example, does not require computation of leakage. storage. or evapotranspiration terms.

Use of disk facilities for storage of array data and interim results

In an effort to expedite use of the program on remote terminals connected to the IBM 370/155, options are included to utilize disk storage facilities. These options enable storage and retrieval of array data (STRT, PERM, and so forth) and the saving of interim head values without punching them on cards.

Use of these options can be particularly beneficial at remote terminals with low speed data transmission or without punch output capability. Also, the type of read statements used afford more efficient data transmission from disk than from cards.

Storage of array data is accomplished via a direct access data set that is defined by a DEFINE FILE statement in the main program (card MAN0480) and by a DD statement in the JCL string used to execute the program. To establish the data set, the DE-FINE FILE statement and the DD statement must indicate the amount of space that is required. The DEFINE FILE statement takes the following form:

DEFINE FILE 2(14,???,U,KKK) **MAN0480**

where ??? is the number of nodes for the problem being solved (DIMLxDIMW). Parameters U and KKK are indicators and do not vary.

The DD statement contains information, such as account number, that will be different for each user. Also, the first reference to the data set is somewhat different from subsequent references. To utilize one of the disk packs provided by the system (IBM 370/ 155) for semipermanent storage of user data. the first reference to the data set will take the following general form if the FORTGCG procedure is used to compile and execute the program.

//GØ. FT02F001 DD DSN = Azzzzz, AZbbb. cxxwwwww.aaaaaaaa,

$$\frac{\text{SPACE} = (????, (?))}{(\text{RECFM} = F)}$$

where

- are the first six digits of a ZZZZZZ nine digit account number: bbb are the last three digits of a nine digit account number:
- is the center code (same as С column 3 on job card):
- is the two digit organization XX code (same as columns 4 and 5 on job card);
- wwwww is the four or five digit program number (same as the program number beginning in column 24 of the job card);
- aaaaaaaa is any 1 to 8 character name used to designate the name of the data set:
- ???? is the number of bytes per record that are to be reserved and should be set equal to DIMLxDIMWx4.

The instructions for the DSN parameter are also given in the CCD users manual. chapter 5, pages 3 and 4. When this initial allocation is processed the system will indicate in the HASP system log, JCL string output, the volume on which the data set was established (for example, SYS011 or SYS015). Subsequent use of the data set must indicate this information by modifying the underlined parameters in the initial reference to the data set. Thus the DD statement will read:

 $//G\emptyset$. FT02F001 DD DSN = Azzzzzz. Azbbb. cxxwwwww.aaaaaaaa,

// UNIT=ØNLINE, DISP=SHR,VØL =SER=yyyyyy

where the DSN parameter is the same as the initial run and yyyyyy indicates the volume (for example, SYS015) on which the data set was established by the initial run. The individual data arrays that are to be stored and later retrieved from this data set are specified on the parameter card for each array. These specifications will be discussed completely in the section on Data Deck Instructions (Attachment III).

If use of this option is selected, space for buffers must be reserved via the REGION parameter on the EXEC card. The amount of space needed is approximately equal to two times the number of bytes per record (indicated in the SPACE parameter on the DD card defined above).

Interim results (head values, cumulative simulation time, and mass-balance parameters) can be punched on cards or can be stored and retrieved from data sets on disk in much the same manner as array data. Use of storage on disk is initiated by parameters on the simulation options card. (See attachment III, card 3.)

Definition of the sequential data set on disk where the information will be stored is accomplished by a DD statement in the JCL string used to execute the program. If one of the system disk packs is used to store the data set, the first reference to the data will be different from subsequent references as in the case of array data sets. The first reference will take the following form if the FORTGCG procedure is used.

- //GØ. FT04F001 DD DSN = Azzzzz. AZbbb. cxxwwww.aaaaaaaaa,
- // UNIT=ØNLINE, DISP=(NEW, KEEP),SPACE=(TRK,(1,1), RLSE),
- // DCB = (RECFM = VBS,LRECL = dddd, BLKSIZE = eeee)

The DSN parameter is defined in the same manner as previously discussed for the direct access (array) data sets and:

dddd—equals DIMLxDIMWx8+48(≤ 6440) eeee—equals DIMLxDIMWx8+52(≤ 6440) If BLKSIZE (eeee) exceeds 6444, code 6444 for (eeee) and 6440 for (dddd). Also, additional core equal to about two times the value of BLKSIZE must be reserved for buffers via the REGION parameter on the EXEC card.

Once the initial reference to the data set has been successfully processed, the system will indicate (via the JCL printout) on what volume the data set has been established (for example, SYS011 or SYS015) and, subsequent references to the data set will appear as follows:

//GØ. FT4F001 DD DSN = Azzzzz.AZbbb. cxxwwww.aaaaaaaaa,

where yyyyyy is the name of the disk pack (for example, SYS011) that contains the data set and DSN is as previously described.

To destroy (erase) an array data set or an interim results data set, simply execute the following job.

// EXEC PGM=IEFBR14

//X DD DSN=Azzzzz.AZbbb.cxxwwww. aaaaaaaa,

// UNIT=ØNLINE, VØL=SER=yyyyyy, DISP=(ØLD,DELETE)

Use of the disk facilities is illustrated in Appendix IV.

Graphical display package

A series of computer programs are currently being written and assembled that will enable graphical display of results of computer models. Components of this graphical display package will include:

- 1. time-series plots of model results on the printer,
- 2. time-series plots on pen plotters (CAL-COMP),
- 3. contour maps of model results at selected time steps on the printer,
- 4. contour maps utilizing pen plotters, and
- 5. other graphical displays, such as perspective (three-dimensional) drawings.

The FORTRAN code shown in figure 27 can be inserted into the program to produce output that can be used in the graphical display package. The changes to MAIN and STEP are required after statements MAN2600 and STP1000, respectively. Statement MAN2600 is deleted. In subroutine PRNTAI, the REAL*8 specification and the DIMENSION statement must be added and the remaining code inserted after statement PRN1650. Also, unit numbers 10 and 11 must be specified on DD statements when the program is executed. Unit 10 is used only for temporary storage and the following DD statement will generally suffice.

- //GØ. FT10F001 DD DSN=&&DATA,DISP = (NEW,DELETE),UNIT =ØNLINE,
- // SPACE = (TRK, (10,5)),DCB =
 (RECFM = VBS,LRECL = 6440,
 BLKSIZE = 6444)

Unit 11 points to the data set that is used to store the data required by the graphical display package and must be semipermanent in nature. That is, it must not be deleted upon completion of your job. The DD statement will generally take the following form.

- //GØ. FT11F001 DD DSN=Azzzzz.AZbbb. cxwwww.aaaaaaaaa,
- // DISP=(NEW,KEEP),UNIT= ØNLINE,SPACE=(TRK,(10,5), RLSE),
- // DCB = (RECFM = VBS, LRECL = 6440, BLKSIZE = 6444)

The data set name parameter (DSN) was discussed in the previous section. The SPACE and DCB parameters shown above should generally be adequate. Recall that once the data set is established, it will be assigned to a certain volume (disk pack) by the IBM operating system. Subsequent references to the data set must include this volume number in the DD statement, that is, $V\emptyset L = SER = ??$.

Results of using a preliminary version of the graphical display package are shown in figures 28 and 29. The time-series plot shown in figure 28 was made on the line printer and the contour map shown in figure 29 was made on a CALCOMP plotter. Documentation on the use of the graphical display package is currently being written.

Modification of program logic

Some users may wish to compile only one or two numerical options with the program. This is done by removing the SOLVE routine(s) not needed from the card deck and modifying the main program in either of the following ways, assuming for this example that SIP is being removed: (1) remove the three IF statements that call SOLVE1, ITER1, and NEWITA, or (2) punch a C in column 1 of these statements and leave them in the main program.

Other modifications to the program logic will be required for certain applications. Modifications will range from changing a few statements to adding a subroutine or deleting options not used. In any case the changes should be made by a programmer familiar with the computational scheme because almost any change has an unanticipated effect on another part of the program requiring several debugging runs.

Reasonably simple modifications to the program include changing format statements and shifting data sets (for example, recharge rate) from GROUP III to GROUP IV so they can be modified for each pumping period.

Adding a second confining bed would be a more complex modification because it may require additional arrays, and ENTRY CLAY in subroutine COEF would have to be made general to accept confining-bed parameters for either bed.

FORTRAN IV

The program includes several FORTRAN IV features that are not in ANS FORTRAN (for example, ENTRY, END parameter in read statement, mixed-mode expressions, G format code, literal enclosed in apostrophes). If the program is used at a computer center where the FORTRAN compiler does not include these extensions, programmers at the

MAIN

```
300 CALL GRAPH (Y(L(3)),Y(L(4)),Y(L(5)),Y(L(6)),Y(L(7)),

1 YY(1),Y(L(8)))

READ (R, 320,END=310) NEXT
```

STEP

WRITE(10) PHI,SUM

PRINTAI

```
REAL#8 HD
```

```
DIMENSION NN(1), SUMX(1), SUMY(1), X(1), Y(1), ZZ(1), HD(1)
C**********
      ENTRY GRAPH
                    (SUMX, SUMY, X, Y, ZZ, HD, NN)
C********
C COMPUTE X AND Y COORDINATES OF ROWS AND COLUMNS
      SUMX(1)=DELX(1)/2.
      SUMY(1)=DELY(1)/2.
      DO 325 I=2.DIML
  325 SUMY(I)=SUMY(I+1)+(DELY(I)+DELY(I+1))/2.
      DO 330 I=2.DIMW
  330 SUMX(I)=SUMX(I+1)+(DELX(I)+DELX(I+1))/2.
C
  DETERMINE NUMBER OF ACTIVE NODES, THEIR STORAGE LOCATION,
  AND THEIR X AND Y COORDINATES
C
      N=0
      DO 340 I=2.IN01
      D0 340 J=2, JN01
      IF (T (I+J) .EQ.0.) GO TO 340
      N=N+1
      NN(N) = I + DIML + (J-1)
      X(N) = SUMX(J)
      Y(N) = SUMY(I)
  340 CONTINUE
  WRITE X AND Y COORDINATES ON UNIT 11
C
      WRITE(11) (X(I), I=1,N)
      WRITE(11) (Y(I), 1=1, N)
  REWIND UNIT 10 AND REPROCESS PHI MATRIX AT EACH TIME STEP
C
   PLACING PHI VALUES AT ACTIVE NODES IN THE ZZ ARRAY (REAL*4)
C
      REWIND 10
      DO 380 I=1+KT
      READ(10) PHI,SUM
      DO 350 J=1.N
      NIJ=NN(J)
  350 ZZ(J) = HD(NIJ)
С
  WRITE PHI VALUES AT ACTIVE NODES AND ELAPSED SIMULATION TIME
  ON UNIT 11
С
      WRITE(11) (ZZ(J) #J=1+N)+SUM
  380 CONTINUE
      WRITE(6,390) N+KT+SUMX(DIMW)+SUMY(DIML)
  390 FORMAT(//+ GRAPHICS OUTPUT FOR +,16, ACTIVE NODES AND +,14,
     1 * TIME STEPS HAS BEEN WRITTEN ON UNIT 11**/*
     2 • MAXIMUM X+Y COORDINATE PAIR IS ++F10.2++++F10.2)
      RETURN
```

FIGURE 27.—Additional FORTRAN code required to produce output for graphical display.



FIGURE 28.—Water level versus time at various nodes of the sample aquifer problem produced by the graphical display package.

selected installation may be available to modify the computer code as necessary.

Limitations of program

The model documented in this report is reasonably free of errors and has been used successfully to simulate a variety of aquifer systems in two dimensions. Undiscovered errors in the logic, however, may appear as the model is applied to a variety of new problems.

The user is cautioned against using this model to make more than a crude simulation of three-dimensional problems. A rigorous analysis of three-dimensional aquifer systems can be made only with the appropriate analog or digital simulators.



FIGURE 29.—Contour map of water level (in feet) for sample aquifer problem produced by graphical display package. Contour interval is 0.5 ft.

Attachment III Data Deck Instructions

Group I: Title, simulation options, and problem dimensions

This group of cards, which are read by the main program, contains data required to dimension the model. To specify an option on card 3, punch the characters underlined in the definition, starting in the first column of the field. For any option not used, leave the appropriate columns blank.

CARD	COLUMNS	FORMAT	VARIABLE	DEFINITION
1	1-80	20A4		Any title the user wishes to print on one
2	1–48	12A4	HEADNG	line at the start of output.
3	1–5	A4,1X	WATER	WATE for water table or combined water-table-artesian aquifer.
	6–10	A4,1X 🗸	LEAK	LEAK for an aquifer system including
	11–15	A4,1X	CONVRT	<u>CONV</u> for combined artesian-water-
	16–20	A4,1X	EVAP	\underline{EVAP} to permit discharge by evapo- transpiration
	21–25	A4,1X	RECH	$\frac{\text{RECH}}{\text{rate.}}$ to include a constant recharge

90	1	LOHNIQUES	OF WAIER-RESC	JURGES INVESTIGATIONS
CARD	COLUMNS	FORMAT	VARIABLE	DEFINITION
	26-30	A4,1X	NUMS	SIP or LSOR or ADI to designate the
				equation-solving scheme.
	31-35	A4,1X	CHCK	CHEC to compute a mass balance.
	36-40	A4,1X	PNCH	$\overline{\text{PUNC}}$ for punched output at the end of
				the simulation.
	41–45	A4,1X	IDK1	DK1 to read initial head and mass bal-
				ance parameters from disk (unit 4).
	46 - 50	A4,1X	IDK2	DK2 to save (write) computed head,
				elapsed time, and mass balance parame-
				ters on disk (unit 4).
	51 - 55	A4,1X	NUM	NUME to print drawdown in numeric
				form.
	56–6 0	A4,1X	HEAD	$\underline{\text{HEAD}}$ to print the head matrix.
		(All	l variables on car	d 4 are integers)
4	1-10	I10	\mathbf{DIML}	Number of rows.
	11 - 20	I10	DIMW	Number of columns.
	21-30	I10	NŴ	Number of pumping wells for which
				drawdown is to be computed at a
				"real" well radius.
	31-40	I10	ITMAX	Maximum number of iterations per time
				step.

NOTE.—Steady-state simulations often require more than 50 iterations. Transient time steps usually require less than 30 iterations.

Group II: Scalar parameters

The parameters required in every problem are underlined. The other parameters are required as noted; when not required, their location on the card can be left blank. The G format is used to read E, F and I data. Minimize mistakes by always right-justifying data in the field. If F format data do not contain significant figures to the right of the decimal point, the decimal point can be omitted. *Default typing of variables applies*.

CARD	COLUMNS	FORMAT	VARIABLE	DEFINITION
1	1-4	A4	CONTR	<u>CONT</u> to generate a map of drawdown and (or) hydraulic head; for no maps insert a blank card.
	11–20	G10.0	XSCALE	Factor to convert model length unit to unit used in X direction on maps (that is, to convert from feet to miles, XSCALE = 5280).
	21-30	G10.0	YSCALE	Factor to convert model length unit to unit used in Y direction on maps.
	31-40	G10.0	DINCH	Number of map units per inch.
	41–50	G10.0	FACT1	Factor to adjust value of drawdown printed*.
	51-60	G10.0	FACT2	Factor to adjust value of head printed*.
	•Value of drawdown or head		FACT 1 or FACT 2	Printed value
	52.57		.01	0 5
			1 10 100	52 525

50

	-					
CARD	COLUMNS	FORMAT	VARIABLE	DEFINITION		
	6168	A8	MESUR	Name of map length unit.		
2	1–10	G10.0	NPER	Number of pumping periods for this simulation.		
	11-20	G10.0	KTH	Number of time steps between printouts.		
	Not	E.—To print o	only the results fo	or the final time step in a pump-		
	ing pe steps.	eriod, make K The program	TH greater than always prints the	the expected number of time results for the final time step.		
	21-30	G10.0	ERR	Error criterion for closure (L) .		
NOTE.—When the head change at all nodes on subsequent itera- tions is less than this value (for example, 0.01 foot), the program has reached a solution for the time step.						
	31-40	G10.0	EROR	Steady-state error criterion (L) .		
NOTE.—If the head change between time steps in transient simula- tions is less than this amount, the pumping period is terminated.						
	41–50	G10.0	SS	Specific storage of confining bed $(1/L)$.		
	Not leakag	re.—SS has a re is a functior	finite value only of storage in the	in transient simulations where confining bed.		
	51-60	G10.0	QET	Maximum evapotranspiration rate (L/T) .		
	61–70	G10.0	ETDIST	Depth at which ET ceases below land surface (L) .		
	No1 evapot	E.—QET and transpiration.	ETDIST required	d only for simulations including		
	71–80	G10.0	LENGTH	Definition depends on the numerical solu- tion used:		
3	1–10	G10.0	HMAX	 LSOR: number of LSOR iterations between 2-D corrections. ADI and SIP: Number of iteration parameters; unless the program is modified, code 10 for SIP. Definition depends on numerical solution 		
			· <u>·</u>	used: LSOR: acceleration parameter.		
				ADI: maximum iteration parameter. SIP: value of β' .		
	N01 inforn	E.—See the dination on itera	scussion of the nution parameters.	umerical methods in the text for		
	11 00	010 0				

11 - 20	G10.0	FACTX	Multiplication factor for transmissivity
			in X direction.
21–30	G10.0	FACTY	Multiplication factor for transmissivity in Y direction

NOTE.—FACTX = FACTY = 1 for isotropic aquifers.

1

CARD	COLUMNS	FORMAT	VARIABLE	DEFINITION
4 5	1-20 21-40 41-60 61-80 1-20	G20.10 G20.10 G20.10 G20.10 G20.10	SUM SUMP PUMPT CFLUXT QRET	Parameters in which elapsed time and cumulative volumes for mass balance are stored. For the start of a simula- tion insert three blank cards. For con- tinuation of a previous run from
	21-40 41-60 61-80	G20.10 G20.10 G20.10	CHST CHDT FLUXT	blank cards and insert the first three cards of the punched output from the
6	120 2140 4160	G20.10 G20.10 G20.10	STORT ETFLXT FLXNT	previous run. If continuation is from interim storage on disk, the three blank cards should remain.

TECHNIQUES OF WATER-RESOURCES INVESTIGATIONS

Group III: Array data

Each of the following data sets, except the first one (PHI), consists of a parameter card and, if the data set contains variable data, may include a set of data cards. *Default typing applies except for* M(I,J) *which is a real array*. Each parameter card contains five variables defined as follows:

COLUMNS	FORMAT	VARIABLE	DEFINITION
1–10 - 1.	G10.0	FACT	 If IVAR=0, FACT is the value assigned to every element of the matrix; If IVAR=1, FACT is the multiplication factor for the following set of data cards.
11–20	G10.0	IVAR	= 0 if no data cards are to be read for this matrix;
			=1 if data cards for this matrix follow.
21-30	G10.0	IPRN	= 0 if input data for this matrix are to be printed;
			= 1 if input data for the matrix are <i>not</i> to be printed.
31–40	G10.0	IRECS	= 0 if the matrix is being read from cards or if each element is being set equal to FACT.
			=1 if the matrix is to be read from disk (unit 2).
41–50	G10.0	IRECD	 = 0 if the matrix is not to be stored on disk. = 1 if the matrix being read from cards or set equal to FACT is to be stored on disk (unit 2) for later retrieval.
	COLUMNS 1-10 A. 11-20 21-30 31-40 41-50	COLUMNS FORMAT 1-10 G10.0 a. 11-20 G10.0 21-30 G10.0 31-40 G10.0 41-50 G10.0	COLUMNS FORMAT VARIABLE 1-10 G10.0 FACT a. 11-20 G10.0 IVAR 21-30 G10.0 IPRN 31-40 G10.0 IRECS 41-50 G10.0 IRECD

Refer to the examples in figures 31–33, Attachment IV. Figure 33 illustrates data for the sample problem without using disk files.

For the uniform starting head = 100, FACT = 100, IVAR = IPRN = IRECS = IRECD = 0and no data cards are required. The storage coefficient matrix is used to locate a constanthead boundary; therefore, FACT = -1, IVAR = 1, IPRN = IRECS = IRECD = 0 and a set of data cards with the location of the boundary nodes follows. To save the storage coefficient matrix on disk (provided unit 2 has been defined on a DD statement; see technical information), set FACT = 1, IVAR = 1, IPRN = IRECS = 0, IRECD = 1, and include the set of data cards (figure 31). After this has been processed successfully, subsequent runs need only include a parameter card with the following: FACT = IVAR = IPRN = 0, IRECS = 1, IRECD = 0. The set of data cards are not included and the storage coefficient matrix is input via unit 2 from disk storage. (See figure 32.)

When data cards are included, start each row on a new card. To prepare a set of data cards for an array that is a function of space, the general procedure is to overlay the finite-difference grid on a contoured map of the parameter and record the average value of the parameter for each finite-difference block on coding forms according to the appropriate format. In general, record only significant digits and no decimal points (except for data set 2); use the multiplication factor to convert the data to their appropriate values. For example, if vertical conductivity of the confining bed (RATE) ranges from 2×10^{-9} to 9×10^{-8} ft/sec, coded values should range from 2 to 90; the multiplication factor (FACT) would be 1.0 E - 9.

Arrays needed in every simulation are underlined. *Omit* parameter cards and data cards *not* used in the simulation (however, see the footnote for the S matrix).

CARD	COLUMNS	FORMAT	VARIABLE	DEFINITION					
1	1-80	8F10.4	PHI(I,J)	Head values for continuation of a previous run (L) .					
	Nor clude a	E.—For a ne . parameter ca	w simulation this ard with this data	data set is omitted. Do not in- set.					
2 3	1–80 1–80	8F10.4 20F4.0	$\frac{\text{STRT}(I,J)}{S(I,J)}$	Starting head matrix (L) . Storage coefficient (dimensionless).					
	Nor: cient v stant-h nodes. probler values,	E.—Always r ralues for art lead boundari At these nod n with no co insert a blan	equired. In additi esian aquifers, thi ies by coding a neg es T or PERM mu onstant-head node k parameter card.	on to specifying storage coeffi- is matrix is used to locate con- gative number at constant-head ust be greater than zero. For a s and that does not require S					
4	1-80	1-80 20F4.0 T(I,J) Transmissivity (L^2/T) .							
	NOT	E.—(1) Requ (2) Zero the con au mo	tired for artesian a values must be p e T or PERM mat mputational schem tomatically insert odel.	aquifer simulation only. blaced around the perimeter of rix for reasons inherent in the ne. If $IVAR=0$, zero values are ted around the border of the					
5	1-80	20F4.0	F4.0 PERM(I,J) Hydraulic conductivity (L/T) (see not 2 for data set 4).						
6	1-80	20F4.0 BOTTOM (I,J) Elevation of bottom of aquifer (L) .							
7	1-80	20F4.0	SY(I,J)	Specific yield (dimensionless).					
	Not bined a	E.—Data sets artesian-wate	s 5, 6, and 7 are re r table simulations	equired for water table or com- s.					
8	1-80	20F4.0	TOP (I,J)	Elevation of top of aquifer (L) .					
	Note	-Required or	ly in combined ar	tesian-water-table simulations					

DATA SET	COLUMNS	FORMAT	VARIABLE	DEFINITION
9	1-80	20F4.0	RATE (I,J)	Hydraulic conductivity of confining bed (L/T) .
10	180	20F4.0	RIVER (I,J)	Head on the other side of confining bed (L) .
11	1-80	20F4.0	M (I,J)	Thickness of confining bed (L) .
	leakag entire and RI be initi	e. If the conf aquifer use t VER do not ialized to a ur	ining bed or strea he M matrix to loc vary over the exte hiform value.	mbed does not extend over the eate the confining bed. If RATE nt of the confining bed they can
12	1-80	20F4.0	GRND (I,J)	Land elevation (L).
	N	оте.—Requir	red for simulations	s with evapotranspiration.
13	1-80	20F4.0	QRE (I,J)	Recharge rate (L/T) .
			Note.—Omit if	not used.
14	1-80	8G10.0	DELX (J)	Grid spacing in X direction (L) .
15	1-80	8G10.0	DELY (I)	Grid spacing in Y direction (L) .

Group IV: Parameters that change with the pumping period

The program has two options for the simulation period:

- 1. To simulate a given number of time steps, set TMAX to a value larger than the expected simulation period. The program will use NUMT, CDLT, and DELT as coded.
- 2. To simulate a given pumping period, set NUMT larger than the number required for the simulation period (for example, 100). The program will compute the exact DELT (which will be ≦DELT coded) and NUMT to arrive exactly at TMAX on the last time step.

Default typing applies.

CARD	COLUMNS	FORMAT	VARIABLE	DEFINITION
1	1–10 11–20	G10.0 G10.0	KP KPM1	Number of the pumping period. Number of the previous pumping period.

NOTE.—In general KPM1=0 if KP=1

KPM1 = 1 if KP = 2, etc.

This causes the time parameter used in ENTRY CLAY to be set to zero and STRT to be initialized to PHI. However, for continuation of a previous pumping period KPM1=KP, and STRT and the time parameter are not affected.

21-30	G10.0	NWEL	Number of wells for this pumping period.
31-40	G10.0	TMAX	Number of days in this pumping period.
41–50	G10.0	NUMT	Number of time steps.
51-60	G10.0	CDLT	Multiplying factor for DELT.
		NOTE.—1.5 is c	ommonly used.
61–70	G10.0	DELT	Initial time step in hours.

DATA	SET 1		(NWEL ca	urds)
	COLUMNS	FORMAT	VARIABLE	DEFINITION
	1–10	G10.0	1	Row location of well.
	11–20	G10.0	J	Column location of well.
	21–30	G10.0	WELL (I,J)	Pumping rate (L^3/T) , negative for a pumping well.
	3140	G10.0	RADIUS	Real well radius (L).

If NWEL=0 the following set of cards is omitted.

putation of drawdown at a real well radius is to be made.

For each additional pumping period, another set of group IV cards is required (that is, NPER sets of group IV cards are required).

If another simulation is included in the same job, insert a blank card before the next group I cards.

Attachment IV

Sample Aquifer Simulation And Job Control Language

This appendix includes examples of job control language (JCL) for several different runs and an example problem designed to illustrate many of the options in the program. The grid and boundary conditions for the problem are given in figure 25. Figure 30 illustrates in cross section the type of problem being simulated, but note that it is not to scale.

The listing of data with the JCL examples is not on a coding form, but it should not be



FIGURE 30.-Cross section illustrates several options included in the sample problem and identifies the meaning of several program parameters.

difficult to determine the proper location of the numbers since the fields are either 4 or 10 spaces. Zero values have not been coded on the data cards to avoid unnecessary punching.

Figures 31 and 32 illustrate the JCL and data decks for two successive simulations of the sample problem. They are designed to show the use of disk facilities to store array data and interim results. The first run (fig. 31) is terminated after 5 iterations and interim results are stored on the data set specified by the FT04F001 DD statement. Note that arrays S, PERM, DELX, and DELY have been stored in the array data set specified by the FT02F001 DD statement (a 1 appears in column 40 of the parameter card for these arrays). The second run (fig. 32) continues computations from the previous stopping point and calculates a solution. Note that PHI, S. PERM, DELX, and DELY are read from disk storage. The final example (fig. 33) illustrates the JCL and data deck for a run without using the disk files. Following figure 33 is the output for the sample prob-



FIGURE 31.—JCL and data deck to copy some of the data sets on disk, compute for 5 iterations, and store the results on disk.



FIGURE 32.-JCL and data deck to continue the previous run (fig. 31) to a solution.

lem generated using the JCL and problem deck shown in figure 33.

Figures 31 to 33 show that the source cards are being compiled for each run. It is more efficient, of course, to compile the source deck once and store it as a load module on disk. Subsequent runs can use the load module with considerable reduction in cards read, CPU time, and lines printed.



FIGURE 33.-JCL and data deck to simulate the sample problem without using disk files.

33
figure
in
illustrated
deck
data
using
Output
Program

U. S. G. S.

FOR SIMULATION OF GROUND-WATER FLOW FINITE-DIFFERENCE MODEL

JANUARY, 1975

HEAD NUME 1500.000 1.000000 0.99999665-01 1.000000 1.000000 FEET 167E ~ ° S 10 - ----- SAMPLE AQUIFER PROBLEM ----ON ALPHAMERIC MAP: MULTIPLICATION FACTOR FOR X DIMENSION = MULTIPLICATION FACTOR FOR Y DIMENSION = MULTIPLICATION FACTOR FOR NET INCH = MUMBER OF FEET PER INCH = MULTIPLICATION FACTOR FOR DRAWDWN = MULTIPLICATION FACTOR FOR HEAD = WORDS OF Y VECTOR USED = NUMBER OF ROWS = NUMBER OF WERER OF ROWS = NUMBER OF COLUMNS = NUMBER OF COLUMNS = NUMBER OF WEILLS FOR WHICH DRAWDOWN IS COMPUTED AT A SPECIFIED RADIUS = MAXIMUM PERMITTED AUMBER OF ITERATIONS = . NUMBER OF PUMPING PERIODS TIME STEPS BETWEEN PRINTOUTS CHEC 9 I P RECH EVAP LEAK WATE SIMULATION OPTIONS:

59

0.0 0.4000000E-06 10.00000

SPECIFIC STORAGE OF CONFINING BED Evapotranspiration rate Effective depth of et

1.000000 1.000000 100.0000

8 6 .

STARTING HEAD

MULTIPLICATION FACTOR FOR TRANSMISSIVITY IN X DIRECTION IN Y DIRECTION

0.3000000E-02

.

ERROR CRITERIA FOR CLOSURE Steady state Error Criteria

					ļ		MATRI	×						
-	0•0	0•0	0 • 0	0•0	0•0	0*0	0.0	0*0	0•0	0*0	0.0	0•0	0.0	0.0
~	0*0	0*0	0.0	0•0	0.0	0.0	0 •0	0.0	0•0	0*0	0.0	0•0	0.0	0.0
ŝ	0.0	0•0	0•0	0•0	0•0	0*0	0•0	0*0	0•0	0*0	0.0	0•0	0*0	0.0
4	0*0	0•0	0•0	0•0	0 • 0	0.0	0°0	0*0	0•0	0*0	0.0	0•0	0*0	0.0
u n	0 0	0*0	0 • 0	0•0	0.0	0*0	0.0	0.0	0•0	0*0	0*0	0•0	0.0	0.0
÷	0°0	0•0	0 * 0	0•0	0 • 0	0*0	0.0	0 • 0	0•0	0.0	0.0	0.0	0•0	0.0
~	0.0	0•0	0.0	0•0	0*0	0.0	0•0	0*0	0•0	0*0	0.0	0.0	0.0	0.0
æ	0 0	0•0	0•0	0.0	0•0	0*0	0•0	0°0	0•0	0*0	0.0	0.0	-1.00000	0.0
6	0.0	0.0	0 • 0	0•0	0•0	0.0	0.0	-1-00000	-1.00000 -	- 1.00000	.1.00000	-1.00000	-1-00000	0•0
10	0.0	0.0	0 * 0	0.0	0*0	0.0	0.0	0•0	0•0	0•0	0*0	0.0	0.0	0.0

Ľ

STORAGE COEFFICIENT

AGUIFER HYDRAULIC CONDUCTIVITY Matrix

				;				:		
-	0°0	0.0	0.0	0.0	0.0	0•0	0.0	0•0	0•0	0.0
N	0.0 0.400E-02	0.0 0.400E-02	0.0	0.0	0,200E-02	0.200E-02	0.200E-02	0.0	0.400E-02	0.400E-02
m	0.0 0.400E-02	0.0 0.400E-02	0.0 0.400E-02	0.200E-02 0.0	0.200E-02	0.200E-02	0.200E-02	0.400E-02	0.400E-02	0.400E-02
4	0,0 0,400E-02	0.0 0.400E-02	0.0 0.400E-02	0.200E-02 0.0	0.200E-02	0.200E-02	0.400E-02	0.400E-02	0.400E-02	0.400E-02
5	0.0 0.400E-02	0.0 0.400E-02	0.0 0.400E-02	0.400E-02 0.0	0.400E-02	0.400E-02	0.400E-02	0.400É-02	0.400E-02	0.400E-02
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MAXIMUM CHANGE IN HEAD FOR THIS TIME STEP = 32.067

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EXPLANATION

R = CONSTANT HEAD BOUNDARY +++ = Value Exceeded 3 Figures #ULTIPLICATION FACTOR = 0.100

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B, DATAI
Flow chart-Continued



Flow chart—Continued





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Flow chart-Continued



Attachment VI Definition Of Program Variables

_	
Δ	IN DATAI. DUMMY ARRAY (DOES NOT USE CORE SPACE) USED TO
	OBTAIN ADDRESSES OF ARRAY DATA SETS:
ALFA	CORRECTION VECTOR FOR ROWS (LSOR) #
	PARAMETER IN SIP ALGORITHM
B	TC(I-1,J)/DELY(I) (1/T);
BE	PARAMETER IN THOMAS ALGORITHM
BOTTOM	ELEVATION OF THE BOTTOM OF THE AQUIFER (L);
CDLT	MULTIPLYING FACTOR FOR THE TIME STEP:
CHCK	CONTAINS CHARACTER STRING FOR MASS BALANCE OPTIONS
CHK	VECTOR CONTAINING PROBLEM OPTIONS:
CONTR	CONTAINS CHADACTED STRING FOR OPTION TO PRINT
CONTR	CONTAINS CHARACTER STRING FOR OFFICE TO FRINT
CONVERT	MARS UP URANDUM AND/UR HEAU;
	CONTAINS CHARACTER STRING FOR WATER TABLE-ARTESIAN OFTIONS
D	TR(1,J=1)/DELX(J) (1/1);
DON	VECTOR THAT CONTAINS DRAWDOWN VALUES (L) I
DEL	ARRAY USED IN SIP ALGORITHM;
DELT	TIME INCREMENT (T);
DELX	GRID SPACING IN THE X DIRECTION (L) #
DELY	GRID SPACING IN THE Y DIRECTION (L);
DIML	NUMBER OF ROWS;
DIMW	NUMBER OF COLUMNSI
EROR	STEADY STATE EROR CRITERION (L);
FRR	CLOSURE CRITERION (L) :
FTA	ARRAY USED IN SIP AL GORTTHME
ETRIST	DEDTH AT WITCH EVADOTDANSDIDATION CEASES BELOW LAND
210131	SUPERCE ())
ETAR	THAT DADT OF ET COUDCE TEDM TREATED IMDITCITIVE
ETAD	THAT FART OF ET SOURCE TERM TREATED INFELCTIELT
	THAT FART UF ET SUURCE TERM TREATED EARLIGTETS, OPTION,
	CONTAINS CHARACTER STRING FOR EVAPOTRANSFIRATION UPTION
F 	
FACT	SEE EXPLANATION IN GROUP IIII ANRAY DATAT
FACTX	MULTIPLICATION FACTOR FOR TRANSMISSIVITY IN & DIRECTION
FACTY	MULTIPLICATION FACTOR FOR TRANSMISSIVITY IN Y DIRECTION
G	PARAMETER IN THOMAS ALGORITHM
н	TC(I+J)/DELY(I) (1/T) #
GRND	ELEVATION OF LAND SURFACE (L) #
HEAD	CONTAINS CHARACTER STRING FOR OPTION TO PRINT HEAD VALUES
HEADNG	TITLE FOR SIMULATION:
HMAX	MAXIMUM ITERATION PARAMETER (ADI)‡
	ACCELERATION PARAMETER (LSOR) \$
	RETA PARAMETER (STP) I
10	INDICATOR USED TO DETERMINE THE TYPE OF ARRAY DATAS
TEDD	THE PUNCTURE VELLS ARE IN SATURATED PART
A CRIM	OF WATED TARLE ADDITEED!
	- 1 DIMOTING WELL HAS GONE DEVI
TETNAI	- A ALL THE CEEDE EVENT THE LACT.
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	- I LAJI IIME JIEM IN MUMPINU MENIUUI
TEMITOTEML	COLEMIS VANIABLE UMMAI AMMATS PASSED ID DATAL VIA AMMAT
	ENIRT PUINTS
IN	IN DATAIN DUMMY ARRAY TO WHICH NAME IS PASSEDT
INOI	DIML-11
IPRN	SEE EXPLANATION IN GROUP III: ARRAY DATA;
IRECS, IREC	D SEE EXPLANATION IN GROUP III.ARRAY DATA:
IRN	RECORD NUMBER USED FOR DISK STORAGE AND RETRIEVAL OF
	ARRAY DATAS

Definition of program variables-Continued

ITMAX	MAXIMUM NUMBER OF ITERATIONS PER TIME STEP;
TVAR	SEE EXPLANATION IN GROUP IIII ARRAY DATA:
TSUM	THE CUMULATIVE WORDS OF STORAGE USED IN THE Y VECTOR:
TZ.JZ.ETC.	DIMENSIONS OF ARRAYS IN MODEL COMPUTED IN MAIN PROGRAMS
JNO1	
KEEP	HYDRAULTC HEAD AT THE PREVIOUS TIME STEP (1):
KKK	ASSOCIATED VARIABLE IN DEFINE FILE. INDICATES NUMBER OF
	NEXT RECORDS
KOUNT	TTERATION COUNTERS
KP	NUMBER OF THE PUMPING PERIOD:
KPN1	NUMBER OF DESTANDING DERIODI
KT	TIME STEP COUNTERS
KTH .	NUMBED OF THE STEPS DETWEEN DOINTOUTS!
1	VECTOR CONTAINING INITIAL ADDRESS OF ADDAVS!
	CONTAINS CHARACTER STRING FOR LEAKAGE OPTIONS
LEAN	NUMBER OF TTERATION DADAMETERS (STR.ADI)
LENGIN	NUMBER OF ITERATION FARAMETERS (SIFTADIT)
M	THICKNESS OF CONFINING OF STDEAM RED (1):
NOCO	NUMBER OF DUMPING DEDITING OF STREAM DED (L))
	CONTAINS CHARACTER STRING FOR ORTION TO PRIME DRAWDOWNS
NUMT	CONTAINS CHARACTER STRING FOR OFILINE TO FRINT DRAWDUWNY
NW	NUMBER OF TIME STEPST
NW	COMPLETED AT A ADEALA WELL DADTUCA
NUC	NUMPER OF WELLS FOR A RUNDING REDIOD!
NWEL	NUMBER OF WELLS FOR A FUMPING PERIODA
NWR	LOCATION OF WELLS;
PNCH	CUNIAINS CHARACIER SIRING FUR UPILUN TO PUNCH HTURAULIC
•	MEAU VALUES; Dotnted linte number;
	PRINIER UNIT NUMBERT
PAKAM	LICHAILUN MARAMEIEK)
	HYDRAULIC CONDUCTIVITY OF THE AUGIPER (L/T))
	HUDDALLIC HEAD ALLING START OF THE ITERATION (L))
	NTURAULIC NEAD (L);
PU 05 T	MAYTMIM EVADOTDANCOTDATION DATE ///TVP
	PRAIMUM EVAPUTRANJFIRATION RATE (L/T/)
0	DEADED INTT MIMBED:
BADTUS	DEAL WELL DADTIIS // \2
DATE	NEAR WELL HADING LEAN
RAIC	AD STORAM DED ///TIN
	CONTAINS CHARACTER STRING EOR DECHARGE ORTIONI
	CURIAINS CHARACIER SIRING FOR RECHARGE UPITONS
RHOR	STUELI (1717) Vector containing territon rarameters:
	VECTOR CUNTAINING THERATION PARAMETERST
RIVER	ABOVE OD DELOW THE DUNDED ADUTEED (1) 1
D W	ADUVE UN DECUNADES COMPED AGUILEN (E))
FN	WELL AND RECHARGE SUDRE TERM (L/T);
5	CONTAINS CHARACTER STRING FOR SID ORIGINA
SIP	CUNIAINS CHARACIER STRING FOR SIF UPILUNT
3L 61 5 4k	TNITTAL & TDANCIENT LEAKAGE (1/T) 9
SLEAN	INTITAL & TANDIENT LEANAGE (L/T/T
33	SPECIFIC STURAGE OF CONFINING BED (1/L/)
STORE	CUNIAINS EITHER THE STURAGE CUEFFICIENT OR SPECIFIC
	TIELU DEPENDING ON THE TYPE OF AQUIFERT
2181	MTUNAULIC MEAD AT THE BEGINNING OF THE CURRENT
CUDC	FURFING FERIUD (L) J
2082	MUDIFIES STURAGE TERM IN WATER TABLE-ARTESIAN CONVERSION
SUM	IVIAL ELAPSED TIME IN THE SIMULATION (T) T
SUMP	IVIAL LLAPSED LIME IN THE PUMPING PERIOD (1))
20KI	SPECTETC VIELD.
त्र च	JECULELU TIELUT TDANSMISSIVITY // 402/TV1
	TRANSHIJGJIII (LTTC/I)) Hadmonto Avedace de tydeiv e tatyd, i vivti
16	MARMONIL AVENAGE OF ITHELT & 1+1/2+3 (L/1)\$

Definition of program variables-Continued

TEMP	VECTOR FOR TEMPORARY STORAGE OF HYDRAULIC HEAD (L);
TEST	= 0 CLOSURE CRITERION SATISFIED;
	= 1 CLOSURE CRITERION NOT SATISFIED;
TEST2	MAXIMUM CHANGE IN HEAD FOR THE TIME STEP (L)
TEST3	VECTOR CONTAINING THE SUM OF THE ABSOLUTE VALUES
	OF HEAD CHANGES FOR EACH ITERATION (L);
TL	TRANSIENT PART OF LEAKAGE COEFFICIENT (1/T);
TMAX	NUMBER OF DAYS IN THE PUMPING PERIOD (T):
TMTN	MINIMUM VALUE OF DIMENSIONLESS TIME FOR THE CURRENT
-	PUMPING PERIOD:
TOP	ELEVATION OF THE TOP OF THE AQUIFER (L):
TR	HARMONIC AVERAGE OF TZDELX @ T+J+1/2 (1/T):
TT	MAXIMUM VALUE OF DIMENSIONLESS TIME FOR THE CURRENT
	PUMPING PERIOD
u	= 0 EXPLICIT TREATMENT OF TRANSIENT LEAKAGES
•	= 1 IMPLICIT TREATMENT OF TRANSIENT LEAKAGES
U	INDICATES DEFINE FILE RECORD LENGTH SPECIFICATION IN WORDS:
v	ARRAY USED IN SIP ALGORITHMI
VF4	VARIABLE FORMAT FOR PRINTING HEAD AND DRAWDOWN:
WATER	CONTAINS CHARACTER STRING FOR WATER TABLE OPTION:
WELL	WELL DISCHARGE (1 **3/T) :
WR	WELL RADIUS (L)
XI	ARRAY CONTAINING INCREMENTAL HEAD VALUES IN SIP SOLUTION (1):
Y	VECTOR CONTAINING ARRAY STORAGES
YDTM	LENGTH OF AQUIFER IN Y DIRECTION (1).
·	
DEFINITIO	N OF VARIABLES IN CHECKI SUBROUTINE
CFLUX	INFLOW FROM RECHARGE WELLS (L*#3/T)#
CFLUXT	CUMULATIVE VOLUME OF WATER FROM RECHARGE WELLS (L++3):
CHD1	RATE OF OUTFLOW TO CONSTANT HEAD BOUNDARY (L++3/T);
CHD2	RATE OF INFLOW FROM CONSTANT HEAD BOUNDARY (L*+3/T);
CHDT	CUMULATIVE DISCHARGE TO CONSTANT HEAD BOUNDARY (L++3);
CHST	CUMULATIVE VOLUME OF WATER INFLOW FROM CONSTANT
с	HEAD BOUNDARY (L++3);
DIFF	ERROR IN MASS BALANCE (L++3);
ETFLUX	EVAPOTRANSPIRATION RATE (L++3/T);
ETFLXT	CUMULATIVE DISCHARGE BY ET (L*#3);
FLUX	RATE OF LEAKAGE DUE TO GRADIENTS AT THE START
	OF THE PUMPING PERIOD (L++3/T) 8
FLUXS	NET LEAKAGE RATE (L++3/T);
FLXN	RATE OF DISCHARGE BY LEAKAGE (L++3/T) #
FLXNT	CUMULATIVE VOLUME OF WATER DISCHARGED BY LEAKAGE (L++3);
FLXPT	CUMULATIVE VOLUME OF WATER INFLOW FROM LEAKAGE (L++3);
PERCNT	PERCENT ERROR IN CUMULATIVE MASS BALANCE:
PUMP	DISCHARGE FROM WELLS (L++3/T);
PUMPT	CUMULATIVE VOLUME OF WATER DISCHARGED BY PUMPING WELLS (L**3);
QREFLX	RECHARGE RATE (L*#3/T);
QRET	CUMULATIVE VOLUME OF WATER DERIVED FROM RECHARGE (L**3):
STOR	RATE OF CHANGE IN STORAGE FOR THE TIME STEP (L++3/T);
STORT	CUMULATIVE VOLUME OF WATER DERIVED FROM STORAGE (L**3)
SUMR	SUM OF RECHARGE AND DISCHARGE RATES FOR THE TIME STEP (L**3/T) :
TOTL1	CUMULATIVE VOLUME OF WATER FROM ALL SOURCES (L**3);
TOTL2	CUMULATIVE VOLUME OF WATER DISCHARGED FROM THE SYSTEM (L**3);
XNET	NET LEAKAGE RATE FOR A CELL (L**3/T).
DEFINITION	N OF VARIABLES IN THE PRINTAL SUBROUTINE
BI ANK	
	VURIAINS DEANN STADUES!
	NUMBER OF MAD UNITS DED INCH!
DIST	NUMBER OF MAP UNITS PER INCH: LOCATION OF NEXT COLUMN OF NODAL VALUES TO BE PRINTED:

Definition of variables in the PRNTAI subroutine-Continued

FACT1	FACTOR FOR ADJUSTING VALUE OF DRAWDOWN PRINTED:
FACT2	FACTOR FOR ADJUSTING VALUE OF HEAD PRINTED;
ĸ	ADJUSTED VALUE OF DRAWDOWN OR HEAD;
MESUR	NAME OF MAP LENGTH UNIT;
N	INDEX FOR SYMBOLS;
NA	INDICES FOR LOCATING X LABEL;
NC	NUMBER OF BLANKS BEFORE GRAPH;
N1	NUMBER OF LINES PER INCH:
N2	NUMBER OF CHARACTERS PER INCH!
NB	NUMBER OF CHARACTERS PER LINE;
N4	NUMBER OF LINES IN THE PLOT
N8	MAXIMUM NUMBER OF CHARACTERS IN Y DIRECTION;
NXD	NUMBER OF INCHES IN THE X DIMENSION OF PLOTE
NYD	NUMBER OF INCHES IN THE Y DIMENSION OF PLOTE
PRNT	CONTAINS THE ARRANGEMENT OF SYMBOLS FOR EACH LINE:
SPACNG	CONTOUR INTERVAL (L);
SYM	VECTOR CONTAINING SYMBOLS USED IN THE PLOT:
TITLE	TITLE FOR PLOT;
VF1.VF2.VF	3 VARIABLE FORMATS FOR CENTERING PLOT:
XLABEL	LABEL FOR X AXIS;
XN	NUMBERS FOR X AXIS;
XN1	1 INCH/(N1+2);
XSCALE	MULTIPLICATION FACTOR TO CONVERT MODEL LENGTH UNIT
	TO UNIT USED IN X DIRECTION ON MAPS;
XSF	X SCALE FACTOR:
YLABEL	LABEL FOR Y AXISI
YLEN	LOCATION OF NEXT VALUE IN THE COLUMN TO BE PRINTED:
YN	NUMBERS FOR Y AXIS;
YSCALE	MULTIPLICATION FACTOR TO CONVERT MODEL LENGTH UNIT
	TO UNIT USED IN Y DIRECTION ON MAPS!
YSF	Y SCALE FACTOR:
Z	LOCATION OF NEXT LINE TO BE PRINTED.

Attachment VII. Program Listing

С	***************************************	N 10
č	FINITE-DIFFERENCE MODEL MA	N 20
Ċ	FOR MA	N 30
č	SIMULATION OF GROUND-WATER FLOW MA	N 40
č	IN TWO DIMENSIONS MA	N 50
č	MA	N 60
č	BY P. C. TRESCOTT, G. F. PINDER AND S. P. LARSON MA	N 70
č	U. S. GEOLOGICAL SURVEY MA	N 80
С	SEPTEMBER: 1975 MA	N 90
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С	MAIN PROGRAM TO DIMENSION DIGITAL MODEL AND CONTROL SEQUENCE MA	N 110
С	OF COMPUTATIONS MA	N 120
С	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	N 130
С	SPECIFICATIONSI MA	N 140
	REAL #4KEEP+M+HEADNG(32) MA	N 150
	REAL #8PHI+G+BE+TEMP+Z+YY MA	N 160
	INTEGER R+P+PU+DIML+DIMW+CHK+WATER+CONVRT+EVAP+CHCK+PNCH+NUM+HEAD+MA	N 170
	1CONTR+LEAK+RECH+SIP+ADI MA	N 180
С	MA	N 190
	DIMENSION Y(70000), L(37), IFNT1(9), IFNT2(9), IFMT3(9), NAME(99),MA	N 200
	1 YY(1) MA	N 210
	EQUIVALENCE (YY(1),Y(1)) MA	N 220
С	MA	N 230
	COMMON /SARRAY/ VF4(11)+CHK(15) MA	N 240
	COMMON /ARSIZE/ IZ,JZ,IP,JP,IR,JR,JC,JC,IL,JL,IS,JS,IH,IMAX,IMX1 MA	N 250
	COMMON /SPARAM/ WATER, CONVRT, EVAP, CHCK, PNCH, NUM, HEAD, CONTR, EROR, LEMA	N 560
	IAK, RECH, SIP, U, SS, TT, TMIN, ETDIST, QET, ERR, TMAX, CDLT, HNAX, YDIM, WIDTH, MA	N 270
	2NUMS LSOH ADI, DELT, SUM SUMP, SURS, STORE TEST, ETGB, ETGD, FACTX, FACTY, MA	N 280
	SIERR KOUNISIFINAL NUMISKISKPSNPERSKIHSIIMAASLENGIHSNWELSNWSDIMLSDIML	N 290
	4MW+JN01+IN01+R+P+PU+1+J+10R2 MA	N 300
C	MA NATA TENTI (44/140 44.TE 441051 441 34 44/141 44 54 741061 441 344	N 310
	UAIA IFMI/4M(INU44M9ID994MIUEI44MI.3/94M(IM 94M9DX94MIUEI44MI.3)MA	N 320
	11777/ / MA Data Temt2/44/101.44.124427.2.44056441/(5.447.20.4456.1.441) MA	N 340
		N 350
	TATA TEMT3/AH(1H0,4H,T5,44H)AEQ,4H,5/(4H)H4H5Y,144H4EQ,4H5Y) MA	N 360
		N 370
	DATA NAME/284H	N 380
	1 TACHRANSSACHIVITACHY SZWACH ACH AACHUIFACHER HACHYCHA	N 390
	2RA+4HULIC+4H CON+4HDUCT+4HIVIT+4HY +4H +4H +4H A+4HOUTF+4HER B+MA	N 400
	34HASE +4HELEV-4HATIO+4HN +3#4H +4H S-4HPECI-4HFTC +4HYTEL-4MA	N 410
	4HD - 4*4H - 4HAQUI - 4HFFR - 4HTOP - 4HFI FV - 4HATIO - 4HN - 4H - 4HMA	N 420
	5CONF + 4HININ + 4HG BE + 4HD HY + 4HDRAU + 4HLIC + 4HCOND + 4HUCTI + 4HVITY + 3*4H MA	N 430
	6 .4H RIV.4HER H.4HEAD .4*4H .4H C.4HONFI.4HNING.4H BED.4H TMA	N 440
	7HI+4HCKNE+4HSS +2*4H +4H L+4HAND +4HSURF+4HACE +4HELE++4HATIMA	N 450
	80,4HN ,3*4H ,4H ARE,4HAL R,4HECHA,4HRGE ,4HRATE,2*4H / MA	N 460
С	MA	N 470
	DEFINE FILE 2(14,2624,U,KKK) MA	N 480
С	······································	N 490
С	MA	N 500
с	READ TITLE, PROGRAM OPTIONS AND PROGRAM SIZE	N 510
	10 READ (R, 370) HEADNG MA	N 520
	WRITE (P:360) HEADNG MA	N 530
	READ (R,380) WATER,LEAK,CONVRT,EVAP,RECH,NUMS,CHCK,PNCH,IDK1,IDK2,MA	N 540
	INUM+HEAD MA	N 550
	WRITE (P,390) WATER,LEAK,CONVRT,EVAP,RECH,NUMS,CHCK,PNCH,IDK1,IDK2MA	N 560
	I+NUM+HEAD MA	N 570

	IF (NUMS.EQ.CHK(11).OR.NUMS.EQ.CHK(12).OR.NUMS.EQ.CHK(13))	GO TO	D 2MAN	580
	10		MAN	590
	WRITE (P+350)		MAN	600
	STOP		MAN	610
2	0 READ_(R+320) DIML+DIMW+NW+ITMAX		MAN	620
-	WRITE (P+340) DIML+DIMW+NW+ITMAX		MAN	630
ç			MAN	640
C	COMPUTE DIMENSIONS FOR ARRAYS		MAN	650
			MAN	660
			MAN	670
	1M=MAX0(19NW) 7MAX-MAX0(D7M) 07000		MAN	680
	IMAAHMAAV(UIME)UIMW) Tet7-dimeastau		MAN	690
	13124018577.1		MAN	700
	130M=C=131C+1 130M=C=131C+1		MAN	710
	1 (1)=}		MAN	720
	D0 30 1=2.4		MAN	730
	1 (T)=TSUM		MAN	790
٦			MAN	750
5	DO 40 TE5-16		MAN	700
			MAN	790
4			MAN	700
	IF (WATER-NE-CHK(Z)) GO TO 60		MAN	800
	DO 50 I=17+19		MAN	810
	L(I)=ISUM		MAN	820
5	0 ISUM≠ISUM+ISIZ		MAN	830
	IP=DIML		MAN	840
	JP=D1MW		MAN	850
	GO TO 80		MAN	860
6	0 D0 70 I=17,19		MAN	870
-	L(I)=ISUM		MAN	880
1	0 ISUM#ISUM+1		MAN	890
	16-1 ThaT		MAN	900
	JET VERK NE CHRIOIN GO TO 100		MAN	910
0	0 17 (LEANONE (MAN (MA)) 00 10 100		MAN	920
) (T)=TSUN		MAN	930
q	0 ISUM=ISUM+ISI7		MAN	940
-			MAN	950
	JREDING		MAN	970
	GQ TQ 120		MAN	980
10	0 D0 110 I=20,22		MAN	990
-	L(I)=ISUM		MANI	000
11	0 TSUM=TSUM+1		MANI	010
• •	IR=1		MAN1	020
	JR≖1		MAN1	030
12	0 IF (CONVRT.NE.CHK(7)) GO TO 130		MAN1	040
	L(23)=ISUM		MAN1	050
	ISUM=ISUM+ISIZ		MAN]	060
	IC=DIML		MAN1	070
	JC=DIMw		MAN1	080
	GO TO 140		MAN]	090
13	0 L(23)=ISUM		MAN1	100
	120m=120m+1		MAN1	110
			MAN	120
14	ULEI 0 TE (EVAR-NE CHK(4)) 60 TO 150		MAN	140
14	V IT LETHEONEOUNIC// OU IV IDV		MANI	1640
	TSIMETSIMATST7		MANI	160
	IL=DIML		MAN	170
	JL=DIMW		MANI	180
	GO TO 160		MANI	190

150	L(24)=ISUM	MAN1200
	ISUM=ISUM+1	MAN1210
	IL=1	MANIZZO
		MANIZZU
160	TE (NUMS, NE CHE/11)) CO TO 190	MANIZJU
100	IC (NOM3+NE+CAR(II)) GU IU IGU	MAN1240
		MAN1250
		MAN1260
170	ISUM=ISUM+ISIZ	MAN1270
	IS=DIML	MAN1280
	JS=DIMW	MAN1290
	GO TO 200	MAN1300
180	DO 190 I=25,28	MAN1310
• • •		MANIJIO
100		MANISZU
190	10-11 10-11	MAN1330
		MAN1340
	02=1	MAN1350
200	DO 210 I=29,31	MAN1360
	L(I)=ISUM	MAN1370
210	ISUM=ISUM+DIMW	MAN1380
	DO 220 I=32,33	MAN1390
	L(T)=TSUM	MAN1400
220		MANIAUU
220		MAN1410
	L (34/-130M	MAN1420
		MAN1430
		MAN1440
	ISUM=ISUM+2*IH	MAN1450
	IF (MOD(ISUM+2).EG.0) ISUM#ISUM+1	MAN1460
	CONTINUE	MAN1470
230	L(36)=ISUM	MANIARO
	ISUM=ISUM+2#IMAX	MAN1480
	L (37) #TSUM	MANIEGO
		MANISUU
	ISUM=ISUM+IMX1	MAN1510
	WRITE (P+330) ISUM	MAN1520
		MAN1530
	PASS INTIIAL ADDRESSES OF ARRAYS TO SUBROUTINES	MAN1540
	CALL DATAI(Y(L(1)),Y(L(7)),Y(L(8)),Y(L(9)),Y(L(10)),Y(L(11)),Y(L(.	MAN1550
]	(2)) • Y (L (13)) • Y (L (14)) • Y (L (15)) • Y (L (16)) • Y (L (17)) • Y (L (18)) • Y (L (19))	MAN1560
2	• Y (L (20)) • Y (L (21)) • Y (I (22)) • Y (I (23)) • Y (I (24)) • Y (I (29)) • Y (I (32)) • Y	MAN1570
	(34))•Y((35)))	MANIERA
		MANIEOO
•	(1 + 1) + (1 +	MANICOO
	() + (L(10)) + (L(23)) + (L(23)) + (L(30)) + (L(32)) + (L(34)) + (L(35)) + (MANIGUU
6		MAN1610
_	IF (NUMS.EG.CHK(11)) CALL SOLVE1(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y	MAN1620
1	L (L (5)) • Y (L (6)) • Y (L (7)) • Y (L (9)) • Y (L (12)) • Y (L (13)) • Y (L (14)) • Y (L (15))	MAN1630
2	2•Y (L (16)) •Y (L (25)) •Y (L (26)) •Y (L (27)) •Y (L (28)) •Y (L (29)) •Ý (L (31)) •Y (MAN1640
3	3L (32)) • Y (L (33)) • Y (L (37)) • Y (L (10)) • Y (L (11)) • Y (L (24)) • Y (I (19)) • Y (I (2	MAN1650
4	3)) • Y (L (20)) • Y (L (22)) • Y (L (21)))	MAN1660
	TF (NUNC-FO, CHK/12)) CALL COLVER/VI (1)) - V/I (2)) - V/I (3)) - V/I (4)) - V/I	MAN1670
,		MANIGIU
		MANIDOU
4	• (L(10)) • (L(25)) • (L(26)) • (L(27)) • (L(28)) • (L(29)) • (L(31)) • (MAN1690
-	۶L (32)) • ۴ (L (33)) • ۴ (L (37)) • ۴ (L (10)) • ۴ (L (11)) • ۴ (L (24)) • ۴ (L (19)) • ۴ (L (2	MAN1700
4	(L (20)) • Y (L (22)) • Y (L (21)))	MAN1710
	IF (NUMS.EQ.CHK(13)) CALL SOLVE3(Y(L(1)),Y(L(2)),Y(L(3)),Y(L(4)),Y	MAN1720
1	L (L (5)) •Y (L (6)) •Y (L (7)) •Y (L (9)) •Y (L (12)) •Y (L (13)) •Y (L (14)) •Y (L (15))	MAN1730
2	2•Y(L(16))•Y(L(25))•Y(L(26))•Y(L(27))•Y(L(28))•Y(L(29))•Y(L(31))	MAN1740
	$(32) \cdot Y (L (33)) \cdot Y (L (36)) \cdot Y (L (37)) \cdot Y (L (10)) \cdot Y (L (11)) \cdot Y (L (24)) \cdot Y (L (37))$	MAN1750
6	$(2) \cdot Y(1(23)) \cdot Y(1(20)) \cdot Y(1(22)) \cdot Y(1(21)))$	MAN1760
		MAN177A
•		MAN170.
	·····································	MAN1760
4	L (17/) + 1 (L (20)) + 1 (L (21)) + 1 (L (22)) + 1 (L (23)) + 1 (L (24)) + 1 (L (29)) + 1 (L (2))	MAN1790
3		MAN1800

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C C

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		CALL CHECKI(Y(L(1)),Y(L(5)),Y(L(6)),Y(L(7)),Y(L(9)),Y(L(10)),Y(L	(1MAN1810
	1	11))•Y(L(12))•Y(L(13))•Y(L(14))•Y(L(15))•Y(L(17))•Y(L(18))•Y(L(19))MAN1820
	2	2 • Y (L (20)) • Y (L (21)) • Y (L (22)) • Y (L (23)) • Y (L (24)) • Y (L (29)) • Y (L (32)))	MAN1830
	-	CALL PRNTAI (Y (L (1)) + Y (L (8)) + Y (L (9)) + Y (L (12)) + Y (L (14)) + Y (L (29)) + Y	(LMAN1840
	1	1(32)))	MAN1850
c	•	• • • • • • • • • • • • • • • • • • • •	
ř			MAN1870
č			MANIGRO
č			MAN1000
č		- DEAD AND WOLLE DATA FOD COOLDE IT AND ITT	MANIOTO
C		A A A A A A A A A A A A A A A A A A A	MAN1900
			MANI9IU
		CALL ARRAY(Y(L(12)), IFMT3, NAME(1), 2)	MAN1920
		IF (WATER-EG-CHK(2)) GO TO 240	MAN1930
		CALL ARRAY(Y(L(9)),IFMT3,NAME(10),3)	MAN1940
		GO TO 250	MAN1950
	240	CALL ARRAY(Y(L(17)),IFMT1,NAME(19),4)	MAN1960
		CALL ARRAY(Y(L(18)),IFMT2,NAME(28),5)	MAN1970
		CALL ARRAY(Y(L(19)),IFMT3,NAME(37),6)	MAN1980
	250	IF (CONVRT.EQ.CHK(7)) CALL ARRAY(Y(L(23)),IFMT2,NAME(46),7)	MAN1990
		IF (LEAK.NE.CHK(9)) GO TO 260	MAN2000
		CALL APPAY (Y (1 (20)) . TENTI . NAME (55) . 8)	MAN2010
		CALL ADDAY(Y(1)(20)) TEMT2NAME(64).0)	MAN2020
		CALL ARRANTIL (21)/ FIF 012 NAME (77)	MAN2030
		CALL ARRAT(1(((22))))) = = = (2) = (MAN2040
	260	IF $(EVAP \bullet EQ \circ CHK(0))$ CALL ARRATUL(24)) IF MIZINAME (02) (11)	MAN2050
		IF (RECH-EG-CHR(IU)) CALL ARRAY(T(L(IJ))+IFMII+NAME(91/+12)	MANZOJO
		CALL MDAT	MANZUOU
С			MANZORO
C		INITIALIZE TRANSMISSIVITY VALUES IN WATER TABLE PROBLEM===	MANZVOU
		KT=0	MAN2090
		IF (WATER.EG.CHK(2)) CALL TRANS	MANZIUU
С			MANZIIU
С		COMPUTE ITERATION PARAMETERS	MANZIZU
		IF (NUMS.EQ.CHK(11)) CALL ITER1	MANZIJU
		IF (NUMS.EQ.CHK(12)) CALL ITER2	MAN2140
		IF (NUMS.EQ.CHK(13)) CALL ITER3	MAN2150
C			MAN2160
С		INITIALIZE PARAMETERS FOR ALPHAMERIC MAP	MAN2170
		IF (CONTR.EQ.CHK(3)) CALL MAP	MAN2180
С			MAN2190
С		+COMPUTE T COEFFICIENTS FOR ARTESIAN PROBLEM+	MAN2200
-		IF (WATER.NE.CHK(2)) CALL TCOF	MAN2210
С			MAN2220
č		READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIC	D=MAN2230
-	270	CALL NEWPER	MAN2240
С	2.0		MAN2250
č		KT=0	MAN2260
			MAN2270
			MANZZBO
r		1 LKN-0	MAN2290
2			MAN2300
C	204		MAN2310
~	690	I UMLL NEWJIF	MAN2320
C C			MAN2330
C		TE LIEAN EN CUMUDY AND EE NE N Y CALL CLAV	MAN2340
~		IF ILEANOEWOURK(7)OMNUOJJONEOVO/ UALL ULAT	MAN2350
C		CHIER ADDODIATE CONVITON DOVITINE AND COMPLETE SOLVITON	MANZIAN
С		ENICH APPROPRIATE SULUTION ROUTINE AND COMPUTE SULUTION	MAN2370
		IF (NUMBEEVEUMR(II)) CALL NEWITE	MAN23PO
		IF (NUMS+EQ.CHK(12)) CALL NEWITH	MANDOCU
		IF (NUMS+EQ+CHK(I3)) CALL NEWIIC	MANDAOO
С			
С		CHECK FOR STEADY STATE AND PRINT OUTPUT AT DESIGNATED	MANCHIU

C.	TIME STEPS	MAN2420
	CALL STEADY	MAN2430
Ċ		MAN2440
C	LAST TIME STEP IN PUMPING PERIOD ?	MAN2450
-	TE (TETNAL NE.1) GO TO 280	MAN2460
r		MAN2470
ř	CHECK FOR NEW PUMPING PERIOD	MAN2480
C		MAN2490
r	IF (RF-LF-RFER) OU TO ETO	MAN2500
C		MAN2510
С	DISK OUTPUT IF DESIRED	MANZOLU
	IF (IDK2•NE•CHK(15)) GO TO 290	MANZSZU
	CALL DISK	MAN2530
С		MAN2540
С	PUNCHED OUTPUT IF DESIRED	MAN2550
	290 IF (PNCH.NE.CHK(1)) GO TO 300	MAN2560
	CALL PUNCH	MAN2570
С		MAN2580
ĉ	CHECK FOR NEW PROBLEM	MAN2590
Ť	300 READ (R+320+END=310) NEXT	MAN2600
	TE (NEXT.EG.0) GO TO 10	MAN2610
		MAN2620
~	310 310r	
č		MAN2640
č		MAN2650
		MAN2030
Ċ		MAN2630
C		MANZOTU
С		MANZOOU
	320 FORMAT (4110)	MANZOYU
	330 FORMAT ('0',54X, WORDS OF Y VECTOR USED =',17)	MANZTUO
	340 FORMAT ('0',62X, NUMBER OF ROWS = ', 15/60X, NUMBER OF COLUMNS =	+15MAN2710
	1 /AV. ANUMBED AF WELLE FAD WUTCH DRAWDAWAL TE CAMBUTER AT A SPECTE	
	TAXALINGHER OF MELLS FOR MUTCH DRAMDOMN IS COMPUTED AT A SPECIA	IECMAN2720
	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS =1	TECMAN2720 15)MAN2730
	2 RADIUS *', 15, /, 39X, 'MAXIMUM PERMITTED NUMBER OF ITERATIONS =' 350 FORMAT ('-', 36X, 'NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO	IECMAN2720 15)MAN2730 N TMAN2740
	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS =1 350 FORMAT (1-1,36X, NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED:/37X,58(141))	IECMAN2720 15)MAN2730 N TMAN2740 MAN2750
	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS =1 350 FORMAT (1-1,36X, NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED:/37X,58(1#)) 360 FORMAT (11:60X, U. S. G. S.://55X, FINITE-DIFFERENCE MODEL:/65	TECMAN2720 15) MAN2730 NN TMAN2740 MAN2750 5X + MAN2760
	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS =1 350 FORMAT (1-1,36X, NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED'/37X,58(14)) 360 FORMAT (11,60X,10, S, G, S,1//55X,1FINITE-DIFFERENCE MODEL!/65 1FOR'/51X,1SIMULATION OF GROUND-WATER FLOW!//60X,1JANUARY, 1975	FIECMAN2720 15)MAN2730 N TMAN2740 MAN2750 5X.FMAN2760 V/1MAN2770
	 2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS #1,350 FORMAT (1-1,36X, MO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED'/37X,58(14)) 360 FORMAT (11,60X,10, S. G. S.1//55X, FINITE-DIFFERENCE MODEL:/651 1FOR'/51X, SIMULATION OF GROUND-WATER FLOW://60X, JANUARY, 1975(233(14)/10),3244//133(14)) 	FIECMAN2720 DIS) MAN2730 DN TMAN2740 MAN2750 DX • MAN2760 D//] MAN2770 MAN2780
	<pre>2 RADIUS ='+15+/+39X+'MAXIMUM PERMITTED NUMBER OF ITERATIONS ='+ 350 FORMAT ('-'+36X+'NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED'/37X+58('*')) 360 FORMAT ('1'+60X+'U. S. G. S.+//55X+'FINITE-DIFFERENCE MODEL+/65 1FOR'/51X+'SIMULATION OF GROUND-WATER FLOW'//60X+'JANUARY, 1975' 233('*')/'0'+32A4//133('*')) 370 FORMAT (20A4)</pre>	FIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 MAN2760 MAN2760 MAN2780 MAN2790
	<pre>2 RADIUS ='+15+/+39X+'MAXIMUM PERMITTED NUMBER OF ITERATIONS ='+ 350 FORMAT ('-'+36X+'NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED'/37X+58('*')) 360 FORMAT ('1'+60X+'U. S. G. S.+//55X+'FINITE=DIFFERENCE MODEL'/65 1FOR'/51X+'SIMULATION OF GROUND=WATER FLOW'//60X+'JANUARY, 1975' 233('*')/'0'+32A4//133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4+1X))</pre>	FIECMAN2720 D5) MAN2730 MAN2740 MAN2750 5X. MAN2760 MAN2780 MAN2790 MAN2800
	<pre>2 RADIUS ='.I5./.39X.*MAXIMUM PERMITTED NUMBER OF ITERATIONS ='. 350 FORMAT ('-'.36X.*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED'.37X.58('*')) 360 FORMAT ('1'.60X.*U. S. G. S.*//55X.*FINITE-DIFFERENCE MODEL:/65 1FOR'/51X.*SIMULATION OF GROUND-WATER FLOW://60X.*JANUARY, 1975 233('*')/'0'.32A4//133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4.1X)) 390 FORMAT ('-SIMULATION OPTIONS: '.13(A4.4X))</pre>	FIECMAN2720 55) MAN2730 MAN2740 MAN2750 50, MAN2760 MAN2780 MAN2790 MAN2800 MAN2810
	<pre>2 RADIUS ='.I5./.39X.*MAXIMUM PERMITTED NUMBER OF ITERATIONS ='. 350 FORMAT ('-'.36X.*NO EQUATION SOLVING SCHEME SPECIFIED. EXECUTIO 1ERMINATED'.37X.58('*')) 360 FORMAT ('1'.60X.*U. S. G. S.*//55X.*FINITE-DIFFERENCE MODEL:/65 1FOR'/51X.*SIMULATION OF GROUND-WATER FLOW://60X.*JANUARY, 1975 233('*')/'0'.32A4//133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4.1X)) 390 FORMAT ('-SIMULATION OPTIONS: '.13(A4.4X)) FAND</pre>	FIECMAN2720 55) MAN2730 MAN2740 MAN2750 55. MAN2760 MAN2760 MAN2780 MAN2790 MAN2800 MAN2810 MAN2820-
	<pre>2 RADIUS #1.15./.39X. MAXIMUM PERMITTED NUMBER OF ITERATIONS =1. 350 FORMAT (1-1.36X. MO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED.37X.58(1*)) 360 FORMAT (11.60X.U. S. G. S.1//55X.FINITE-DIFFERENCE MODEL1/65 1FOR'/51X.SIMULATION OF GROUND-WATER FLOW1//60X.JANUARY, 1975 233(1*1)/101.3244//133(1*)) 370 FORMAT (2044) 380 FORMAT (16(A4.1X)) 390 FORMAT (1-SIMULATION OPTIONS: 1.13(A4.4X)) END</pre>	FIECMAN2720 515) MAN2730 MAN2740 MAN2750 51. MAN2760 7/1MAN2760 MAN2780 MAN2790 MAN2800 MAN2810 MAN2820-
	<pre>2 RADIUS #*+I5+/+39X+*MAXIMUM PERMITTED NUMBER OF ITERATIONS =*+ 350 FORMAT (*-*+36X+*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED*/37X+58(***)) 360 FORMAT (*1++60X+*U+ S+ G+ S+*//55X+*FINITE=DIFFERENCE MODEL*/65 1FOR*/51X+*SIMULATION OF GROUND=WATER FLOW*//60X+*JANUARY+ 1975* 233(***)/*0*+32A4//133(***)) 370 FORMAT (20A4) 380 FORMAT (16(A4+1X)) 390 FORMAT (*-SIMULATION OPTIONS: *+13(A4+4X)) END</pre>	FIECMAN2720 DIS) MAN2730 DN TMAN2740 MAN2750 DX MAN2760 MAN2760 MAN2780 MAN2780 MAN2790 MAN2800 MAN2810 MAN2820-
	2 RADIUS ***I5*/*39X**MAXIMUM PERMITTED NUMBER OF ITERATIONS *** 350 FORMAT (*-**36X**NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED*/37X*58(***)) 360 FORMAT (*1**60X**U********************************	FIECMAN2720 DIS MAN2730 DN TMAN2740 MAN2750 DX MAN2760 V/IMAN2770 MAN2780 MAN2780 MAN2790 MAN2800 MAN2810 MAN2820- DTTCDAT 10
	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS #1, 350 FORMAT (1-1,36X, NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED'/37X,58(1#1)) 360 FORMAT (11,60X,1U, S. G. S.1//55X, FINITE-DIFFERENCE MODEL:/65 1FOR'/51X,'SIMULATION OF GROUND-WATER FLOW'//60X,'JANUARY, 1975 233(1*1)/10',32A4//133(1*1)) 370 FORMAT (20A4) 380 FORMAT (16(A4,1X)) 390 FORMAT (16(A4,1X)) 390 FORMAT ('-SIMULATION OPTIONS: 1,13(A4,4X)) END SUBROUTINE DATAI(PHI+STRT+SURI+Ť+TR+TC+S+GRE+WELL+TL+SL+PERM+BO 1M,SY+RATE+RIVER+M+TOP,GRND+DELX+DELY+WR+NWR)	FIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 MAN2760 MAN2760 MAN2780 MAN2790 MAN2800 MAN2810 MAN2820- DTTCDAT 10 DAT 20
с	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS #1, 350 FORMAT (1-1,36X, NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED'/37X,58(1*1)) 360 FORMAT (11,60X,1U, S. G. S.1//55X, FINITE-DIFFERENCE MODEL:/65 1FOR'/51X, SIMULATION OF GROUND-WATER FLOW://60X, JANUARY, 1975 233(1*1)/10',32A4//133(1*1)) 370 FORMAT (20A4) 380 FORMAT (16(A4,1X)) 390 FORMAT (16(A4,1X)) 390 FORMAT (1-SIMULATION OPTIONS: 1,13(A4,4X)) END SUBROUTINE DATAI(PHI,STRT,SURI,Ť,TR,TC,S,GRE,WELL,TL,SL,PERM;BC 1M,SY,RATE,RIVER,M,TOP,GRND,DELX;DELY,WR,NWR)	FIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 MAN2760 MAN2760 MAN2780 MAN2790 MAN2800 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 30
CC	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS =1 2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS =1 350 FORMAT ('-',36X, NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED'/37X,58('*')) 360 FORMAT ('1',60X, U. S. G. S. 1//55X, 'FINITE-DIFFERENCE MODEL'/69 1FOR'/51X, 'SIMULATION OF GROUND-WATER FLOW'//60X, 'JANUARY, 1975' 233('*')/'0',32A4//133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4,1X)) 390 FORMAT (16(A4,1X)) 390 FORMAT ('-SIMULATION OPTIONS: ',13(A4,4X)) END SUBROUTINE DATAI(PHI,STRT,SURI,T.TR,TC,S,GRE,WELL,TL,SL,PERM,BO 1M,SY,RATE,RIVER,M,TOP,GRND,DELX;DELY,WR,NWR) READ AND WRITE INPUT DATA	FIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 MAN2760 MAN2760 MAN2780 MAN2780 MAN2800 MAN2820- DITCDAT 10 DAT 20 DAT 30 DAT 40
C C C C	2 RADIUS ***I5*/*39X**MAXIMUM PERMITTED NUMBER OF ITERATIONS =** 350 FORMAT (*-**36X**NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED************************************	TIECMAN2720 515) MAN2730 MAN2740 MAN2750 51, MAN2760 7/1 MAN2760 MAN2770 MAN2780 MAN2820 MAN2820 MAN2820 0 MAN2820 0 MAN270 0 MAN270
0000	2 RADIUS ***I5*/*39X**MAXIMUM PERMITTED NUMBER OF ITERATIONS =** 350 FORMAT (*-**36X**NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED*/37X*58(***)) 360 FORMAT (****60X**U******************************	FIECMAN2720 DISJMAN2730 DN TMAN2740 MAN2750 MAN2760 MAN2760 MAN2780 MAN2780 MAN2780 MAN2800 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 30 DAT 40 DAT 50 DAT 60
00000	2 RADIUS ***I5*/*39X**MAXIMUM PERMITTED NUMBER OF ITERATIONS =** 350 FORMAT (*-**36X**NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED*/37X*58(***)) 360 FORMAT (*1**60X**U***) S. G. S.**//55X**FINITE=DIFFERENCE MODEL*/65 1 FOR*/51X**SIMULATION OF GROUND=WATER FLOW*//60X**JANUARY**1975* 233(***)/*0**3244//133(***)) 370 FORMAT (2044) 380 FORMAT (16(A4*1X)) 390 FORMAT (16(A4*1X)) 390 FORMAT (*-SIMULATION OPTIONS: **13(A4***)) END SUBROUTINE CATAI(PHI*STRT*SURI***TR*TC*S*GRE*WELL*TL*SL*PERM*80 1M*SY*RATE*RIVER*M*TOP*GRND*DELX*DELY*WR*NWR) 	FIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 DX TMAN2760 V/IMAN2770 MAN2780 MAN2780 MAN2780 MAN2800 MAN2800 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 40 DAT 50 DAT 60 DAT 70
00000	2 RADIUS ***I5*/*39X**MAXIMUM PERMITTED NUMBER OF ITERATIONS =** 350 FORMAT (*-**36X**NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED*/37X*58(***)) 360 FORMAT (*1**60X**U***) S. G. S.**//55X**FINITE=DIFFERENCE MODEL*/65 1 FOR*/51X**SIMULATION OF GROUND=WATER FLOW*//60X**JANUARY**1975* 233(***)/*0**3244//133(***)) 370 FORMAT (2044) 380 FORMAT (16(A4*1X)) 390 FORMAT (16(A4*1X)) 390 FORMAT (*-SIMULATION OPTIONS: **13(A4***)) END SUBROUTINE DATAI(PHI*STRT*SURI***TR*TC*S*GRE*WELL*TL*SL*PERM*B(1M*SY*RATE*RIVER*M*TOP*GRND*DELX*DELY*WR*NWR) 	FIECMAN2720 DIS) MAN2730 DN TMAN2740 MAN2750 DX MAN2760 V/IMAN2770 MAN2780 MAN2780 MAN2780 MAN2800 MAN2810 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 40 DAT 50 DAT 60 DAT 70 DAT 80
00000	2 RADIUS ***I5*/*39X**MAXIMUM PERMITTED NUMBER OF ITERATIONS *** 350 FORMAT (*-**36X**NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED*/37X*58(***)) 360 FORMAT (*1**60X**U********************************	FIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 DX TMAN2760 V/IMAN2770 MAN2780 MAN2780 MAN2790 MAN2800 MAN2810 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 40 DAT 60 DAT 60 DAT 80 DAT 90
00000	2 RADIUS #1,15,/,39X, MAXIMUM PERMITTED NUMBER OF ITERATIONS =1, 350 FORMAT ('-',36X,*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED'/37X,58(**')) 360 FORMAT ('1'+60X,*U, S. G. S.*//55X,*FINITE=DIFFERENCE MODEL*/65 1 FOR'/51X,*SIMULATION OF GROUND=WATER FLOW*//60X,*JANUARY, 1975 233('*')/'0',32A4//133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4+1X)) 390 FORMAT (16(A4+1X)) END SUBROUTINE DATAI(PHI,STRT,SURI,Ť,TR,TC,S,GRE,WELL,TL,SL,PERM,BO 1M,SY,RATE,RIVER,M,TOP,GRND,DELX;DELY,WR,NWR) READ AND WRITE INPUT DATA SPECIFICATIONS: REAL *8PHI,DBLE,XLABEL,YLABEL,TITLE,XN1,MESUR REAL *4M INFEGER B,P,PUL,DIMI,DIMM,CHK,WATER,CONVET,EVAP,CHCK,PNCH,NUM,HI	TIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 MAN2760 MAN2760 MAN2760 MAN2780 MAN2790 MAN2800 MAN2800 MAN2810 MAN2820- DITTCDAT 10 DAT 20 DAT 30 DAT 40 DAT 50 DAT 60 DAT 70 DAT 80 DAT 90 EAD, DAT 100
00000	2 RADIUS ***I5./*39X**MAXIMUM PERMITTED NUMBER OF ITERATIONS =** 350 FORMAT (*-*:36X**NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED ***I5.*********************************	FIECMAN2720 DIS) MAN2730 MAN2740 MAN2750 MAN2760 MAN2760 MAN2760 MAN2780 MAN2790 MAN2800 MAN2800 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 30 DAT 40 DAT 50 DAT 40 DAT 80 DAT 80 DAT 90 EAD, DAT 100 DAT 110
00000 0	<pre>1/94.*NUMBER OF WELLS FOR WHICH DRAWUMEN IS COMPOLED AT A SPECIF 2 RADIUS #1,15,/,39X,*MAXIMUM PERMITTED NUMBER OF ITERATIONS #1 350 FORMAT ('-',36X,*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED'/37X,58(**')) 360 FORMAT ('1'*60X,*U. S. G. S.*//55X,*FINITE=DIFFERENCE MODEL'/69 1FOR'/51X,*SIMULATION OF GROUND-WATER FLOW*//60X,*JANUARY, 1975 233(**)/'0',3244/133(**')) 370 FORMAT (2044) 380 FORMAT (16(44,1X)) 390 FORMAT (16(44,1X)) END SUBROUTINE DATAI(PHI+STRT+SURI+Ť+TR+TC+S+ORE+WELL+TL+SL+PERM+BO 1M,SY+RATE,RIVER,M+TOP,GRND,DELX;DELY+WR+NWR) </pre>	TIECMAN2720 DISIMAN2730 MAN2740 MAN2750 MAN2760 MAN2760 MAN2760 MAN2760 MAN2700 MAN2800 MAN2800 MAN2820- DITCDAT 10 DAT 20 DAT 30 DAT 40 DAT 50 DAT 60 DAT 70 DAT 80 DAT 90 EAD,DAT 100 DAT 120
ссссс С	2 RADIUS #1,15,1,39X,*MAXIMUM PERMITTED NUMBER OF ITERATIONS #1 350 FORMAT ('-',36X,*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED'/37X,58('*')) 360 FORMAT ('1',60X,*U. S. G. S.*//55X,*FINITE=DIFFERENCE MODEL:/65 1 FOR'/51X,*SIMULATION OF GROUND=WATER FLOW*//60X,*JANUARY, 1975 233('*')/'0',32A4//133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4,1X)) 390 FORMAT ('-SIMULATION OPTIONS: ',13(A4,4X)) END SUBROUTINE CATAI(PHI+STRT,SURI.†*TR*TC+S*GRE*WELL*TL*SL*PERM*BO 1M,SY*RATE*RIVER*M*TOP*GRND*DELX;DELY*WR*NWR) 	FIECMAN2720 DIS MAN2720 DIS MAN2730 MAN2740 MAN2750 DIS MAN2760 MAN2760 MAN2780 MAN2780 MAN2780 MAN2800 MAN2800 MAN2810 MAN2810 MAN2820- DITCDAT 10 DAT 20 DAT 40 DAT 20 DAT 40 DAT 50 DAT 60 DAT 70 DAT 80 DAT 90 EAD, DAT 100 DAT 120 Z, JZ0AT 130
00000 C	<pre>1/94.*NUMBER OF WELLS FOR WHICH DRAWOWN IS COMPUTED AT A SPECIF 2 RADIUS #1,15,/39X,*MAXIMUM PERMITTED NUMBER OF ITERATIONS #1 350 FORMAT (1-1,36X,*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1ERMINATED'/37X,58(**')) 360 FORMAT (11*60X,*U. S. G. S.*//55X,*FINITE-DIFFERENCE MODEL*/65 1FOR'/51X,*SIMULATION OF GROUND-WATER FLOW*//60X,*JANUARY, 1975 233(**)/*0*,3244/133(***)) 370 FORMAT (2044) 380 FORMAT (2044) 380 FORMAT (16(44+1X)) 390 FORMAT (16(44+1X)) END SUBROUTINE CATAI(PHI*STRT*SURI*Ť*TR*TC*\$,GRE*WELL*TL*SL*PERM*86 1M*SY*RATE*RIVER*M*TOP*GRND*DELX*DELY*WR*NWR) </pre>	FIECMAN2720 DIS) MAN2730 DN TMAN2740 MAN2750 DX MAN2750 DX MAN2760 MAN2760 MAN2780 MAN2780 MAN2780 MAN2800 MAN2800 MAN2800 MAN2800 MAN2800 MAN2820- DTTCDAT 10 DAT 20 DAT 40 DAT 40 DAT 50 DAT 60 DAT 80 DAT 80 DAT 90 EAD, DAT 100 DAT 120 Z, JZDAT 130 (17, DAT 140
00000 C	<pre>1/34************************************</pre>	FIECMAN2720 DIS) MAN2730 DN TMAN2740 MAN2750 DS, MAN2760 P//IMAN2770 MAN2780 MAN2780 MAN2800 MAN2800 MAN2800 MAN2800 MAN2800 MAN2800 MAN2820- DTTCDAT 10 DAT 20 DAT 40 DAT 40 DAT 50 DAT 60 DAT 60 DAT 60 DAT 80 DAT 90 EAD, DAT 100 DAT 120 CAJZDAT 130 (IZ, DAT 140 (IZ, DAT 150
00000 C	<pre>1/94.HUMBER OF WELLS FOR WHICH DRAWDOWN IS COMPUTED AT A SPECIF 2 RADIUS =:,15,/,39X,*MAXIMUM PERMITTED NUMBER OF ITERATIONS =: 350 FORMAT ('-',36X,*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED'/37X,58('*')) 360 FORMAT ('',60X,*U. S. G. S.*//55X,*FINITE-DIFFERENCE MODEL:/65 1FOR'/51X,*SIMULATION OF GROUND-WATER FLOW*//60X,*JANUARY, 1975 233('*')/'0',32A4//133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4+1X)) 390 FORMAT (16(A4+1X)) SUBROUTINE DATAI(PHI,STRT,SURI,*,TR,TC,S,QRE,WELL,TL,SL,PERM,BC 1M,SY,RATE,RIVER,M,TOP,GRND,OELX,DELY,WR,NWR) </pre>	FIECMAN2720 DIS) MAN2730 DN TMAN2740 MAN2750 DS, MAN2760 P//MAN2770 MAN2780 MAN2780 MAN2790 MAN2800 MAN2800 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 30 DAT 40 DAT 50 DAT 60 DAT 60 DAT 70 DAT 80 DAT 90 EAD, DAT 100 DAT 110 DAT 120 Z, JZDAT 130 (IR, DAT 150 (IH) DAT 150
CCCCC C	<pre>1/skindhele OF Wells FOR WHICH DRAWDUM IS COMPOLED AT A SPECIF 2 RADIUS =',15,/,39X,'MAXIMUM PERMITTED NUMBER OF ITERATIONS =' 350 FORMAT ('-',36X,'NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTIO 1ERMINATED'/37X,58('*')) 360 FORMAT ('!'60X,'U. S. G. S.'//55X,'FINITE-DIFFERENCE MODEL'/65 1FOR'/51X,'SIMULATION OF GROUND-WATER FLOW'//60X,'JANUARY, 1975' 233('*')/'0',32A4/133('*')) 370 FORMAT (20A4) 380 FORMAT (16(A4,1X)) 390 FORMAT (16(A4,1X)) 800 FORMAT ('-SIMULATION OPTIONS: '.13(A4,4X)) END SUBROUTINE DATAI(PHI.STRT.SURI.T.TR.TC.S.GRE.WELL.TL.SL.PERM.BO 1M.SY.RATE,RIVER.M.TOP.GRND.DELX.DELY.WR.NWR) </pre>	FIECMAN2720 DIS) MAN2730 DN TMAN2740 MAN2750 DS, MAN2760 D//IMAN2770 MAN2780 MAN2790 MAN2800 MAN2800 MAN2810 MAN2820- DTTCDAT 10 DAT 20 DAT 30 DAT 40 DAT 50 DAT 40 DAT 40 DAT 80 DAT 80 DAT 90 EAD, DAT 100 DAT 120 Z, JZDAT 130 (IZ, DAT 140 (IR, DAT 150 CAT 170
CCCCC C	<pre>1/skindher OF Wells FOR WHICH DRAWDUMN IS CHEMPTED AT A SPECIF 2 RADIUS =*,I5,/39X,*MAXIMUM PERMITTED NUMBER OF ITERATIONS =* 350 FORMAT ('-',36X,*NO EQUATION SOLVING SCHEME SPECIFIED, EXECUTION 1 ERMINATED*/37X,58('*')) 360 FORMAT (''''''''''''''''''''''''''''''''''''</pre>	FIECMAN2720 DIS) MAN2720 DIS) MAN2730 MAN2740 MAN2750 DIS) MAN2760 DIS) MAN2760 DIS) MAN2760 MAN2790 MAN2800 MAN2800 MAN2800 MAN2800 MAN2800 MAN2800 MAN2820- DIS MAN2800 MAN2800 MAN2800 DIS MAN2800 MAN2800 DIS MAN2800 DIS MAN2800 DIS MAN2800 DIS MAN2800 DIS MAN2800 DIS MAN2800 DIS MAN2800 DIS MAN2800 DIS MAN2800 DIS MIN MIN MIN MIN MIN MIN MIN MIN MIN MIN

	COMMON / CARDAN / NEA / 111 - CHK / 151	DAT	100
	COMMON /SARRAT/ VF4(II) (CRK(IS)	UAI	190
	CUMMON /SPAHAM/ WATER, CUNVRITEVAP, CHCR, PNCH, NUMTHEAD, CUNTRIERUR, LI	LUAT	200
	1AK,RECH,SIP,U,SS,TT,TMIN,ETDIST,GET,ERR,TMAX,CDLT,HMAX,YDIM,WIDTH	DAT	210
	2NUMS,LSOR,ADI,DELT,SUM,SUMP,SUBS,STORE,TEST,ETQB,ETQD,FACTX,FACTY	DAT	220
	3IERR,KOUNT,IFINAL,NUMT,KT,KP,NPER,KTH,ITMAX,LENGTH,NWEL,NW,DIML,D	IDAT	530
	4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2	DAT	240
	COMMON /CK/ FTELXT.STOPT.OPET.CHST.CHDT.FLUXT.PUMPT.CELUXT.FLUXT	DAT	250
		OAT.	340
	COMMON /FR/ ALABEL (3) (ILABEL (0) (IIILE (3) (ANI) MESUR (FRNI (IZZ) (BLAN	VUAT	200
	1(60) DIGIT(122) VF1(6) VF2(6) VF3(7) XCALEDINCHSYM(17) XN(100)	DAT	270
	2YN(13),NA(4),N1,N2,N3,YSCALE,FACT1,FACT2	DAT	280
	COMMON /ARSIZE/ IZ,JZ,IP,JP,IR,JR,IC,JC,IL,JL,IS,JS,IH,IMAX,IMX1	DAT	290
	RETURN	DAT	300
С		DAT	310
ř		DAT	320
C	ENTRY DATATA	DAT	220
~		DAT	340
Č,	********	DAT	340
C		UAI	350
Ç	READ AND WRITE SCALAR PARAMETERS	DAT	360
	READ (R+500) CONTR+XSCALE+YSCALE+DINCH+FACT1+FACT2+MESUR	DAT	370
	IF (CONTR.EQ.CHK(3)) WRITE (P,610) XSCALE, YSCALE, MESUR, MESUR, DINC	TADE	380
	1+FACT1+FACT2	DAT	390
	READ (R+490) NPER+KTH+ERR+EROR+SS+QET+ETDIST+LENGTH+HMAX+FACTX+FA	CDAT	400
	177	DAT	410
	TE (ETDIST LE A) ETDIST=1	DAT	420
	AT ALIVISTOLLOVOT LIVISTOL Hotte (D.653) NGEO.KT.500.60.CC.051.5TDICT.5ACTY.5ACTY	DAT	430
	WKITE (FYJEU) NFERYKTRYERKYERKKYERKKYERKYESJWETYETDIJIYI MOTAT MOTI	DAT	4.50
Ċ		DAT	440
С	READ CUMULATIVE MASS BALANCE PARAMETERS	DAL	450
	READ (R+600) SUM+SUMP+PUMPT+CFLUXT+GRET+CHST+CHDT+FLUXT+STORI+ETF	DAI	400
	1XT+FLXNT	DAT	470
	IF (IDK1+EQ+CHK(14)) GO TO 20	DAT	480
	IF (SUM.EQ.0.0) GO TO 40	DAT	490
	WRITE (P+480) SUM	DAT	500
~		DAT	510
		041	510
Ċ		DAT	520
C	HEAD DATA TO CONTINUE PREVIOUS COMPUTATIONS READ HERE	DAT	530
С	FROM CARDS:	DAT	540
	DO 10 I=1,DIML	DAT	550
	READ (R + 540) (PHI(I + J) + J = 1 + DIMW)	DĄT	560
	10 WRITE (P,530) I,(PHI(I,J),J=1,DIMW)	DAT	570
	GC TO 40	DAT	580
c	PEAD AND WRITE DATA FROM UNIT 4 ON DISK PATHER THAN CARDS!	DAT	560
C	20 DEAD (A) DUT SHAR SHAD DIADT CELHYT OFF FUEL CAT ELHYT STODT ETER	DAT	400
	20 REAU (47 FRITSUMTSUMFFUMFITGELUKITGREITENSITERUTTEUKITSIORITEIT		600
		DAT	010
	WRITE (P+480) SUM	UAI	620
	DO 30 I=1+DIML	DAT	630
	30 WRITE (P+530) I,(PHI(I,J)+J=1+DIMW)	DAT	640
	REWIND 4	DAT	650
С		DAT	660
	40 READ (R+490) FACT+IVAR+IPRN+IRECS+IRECD	DAT	670
	IF (1RECS-EQ-1) READ (2+1) STRT	DAT	680
	TE ((IVAR-EQ.)-OR-TRECS-EQ.).AND-TPRN-NE-1) WRITE (P.470)	DAT	690
		DAT	700
	TE (TVAD EG IN DEAD (D.EAG) (CTDT/T.IV. H.P.DTMM)	DAT	710
	17 1140454011 NEAD (K43401 (3(K1(140)40=1401MM)	DAT DAT	710
			120
	IF (IRECS.EG.1) GO TO 60	DAT	730
	IF (IVAR.NE.1) GO TO 50	DAT	740
	STRT(I,J)=STRT(I,J)*FACT	DAT	750
	GO TO 60	DAT	760
	50 STRT(I+J)=FACT	DAT	770
	60 SURI(I+J)=STRT(I+J)	DAT	780
		DAT	705

		TL(I+J)=0+	DAT 790
		$SL(I \cdot J) = 0$	DAT 800
		TR(I,J)=0.	DAT 810
		TC(I,J) = 0.	DAT 820
		wE(1)(1, J) = 0.0	DAT 830
		QBF(I,J)=0.	DAT 840
	70	TF (SUM+EQ+0,0+AND+TDK1+NE+CHK(14)) PHI(I+J)=STRT(I+J)	DAT 850
		IF (IVAR-FQ-0-AND-IRECS-FQ-0-QR-IPRN-EQ-1) GO TO 80	DAT 860
		WETTE (P.530) I.(STRT(I.J),J=1.DIMW)	UAT 870
	60	CONTINUE	DAT 880
		TE (IVAR.NE.1.AND.TECS.NE.1) WRITE (P.420) FACT	DAT 890
		TE (TRECO.EG.1) WEITE (21) STRT	DAT 900
		IT (INCODECCI) WHILE (E'I') STAT	DAT 910
~		AE FORM	DAT 920
Č			DAT 930
c		OBTIONS AND BETTE THEN ON DISK TE SECONDA AND THE OBTIONS	DAT 940
C		UPITONS) AND WRITE THEM ON DISK IF SPECIFIED IN THE OFISONS	DAT 050
C			0AT 060
-		ENTRY ARRAT(A)IPMI()IN()	DAT 900
С			DAT 970
		READ (R,490) FACT, IVAR, IPRN, IRECS, IRECD	DAT 980
		IK=4+IRECS+2+IVAR+IPRN+1	UA1 990
		GO TO (90,90,110,110,140,140), IK	DAILOOD
	90	D0 100 I=1.0IML	DAT1010
		DO 100 J=1+DIMW	DAT1020
	100	A(I+J)=FACT	DAT1030
		WRITE (P+430) IN+FACT	DAT1040
		GO TO 160	DAT1050
	110	IF (IK.EQ.3) WRITE (P.440) IN	DAT1060
	• •	DO 130 I=1+DIML	DAT1070
		READ (R+510) (A(I+J)+J=1+DIMW)	DAT1080
		MWID.120 J=1.01WW	DAT1090
	120	$A(I \bullet J) = A(I \bullet J) \bullet FACT$	DAT1100
	130	IF (IK_EQ.3) WRITE (P.IFMT) I.(A(I.J),J=1.DIMW)	DAT1110
		60 TO 160	DAT1120
	140	READ (2+IRN) A	DAT1130
	• •	IF (IK-EQ.6) GO TO 160	DAT1140
		WRITE (8+440) IN	DAT1150
			DAT1160
	150	WRITE (PEIEMT) I. (A(I.J.).J=1.DIMW)	DAT1170
	160	TE (TRECD_EQ.) WRITE (2)TRN) A	DAT1180
	100	RETIRN	DAT1190
c			DAT1200
č			DAT1210
č		BORDER OF THE MODEL	DAT1220
č			CAT1230
Ċ			DAT1240
~			DAT1250
C			DAT1260
			DAT1270
		DO 100 0-1901000 TE (WATER EQ.CHK(2)) GO TO 170	DAT1280
		$T = \{T = \{0, 1, 0\}, T \in \{0, 1\}, 0\}, 0 = \{1, 0\}, 1 = \{0, 1\}, 0\}, T = \{T, 1\}, 0\}$	DAT1290
			DATIBOO
	170	TE (T.FO.).00.T.FO.DINI.00.1.FO.1.00.1.FO.DINW) PEPM(T.1)=0	
	180	CONTINUE	DAT1320
r	190		0411320
Ļ		05A0 / D.AGO / FAT. TVAD. TOPN. TOFC. TOFON	DAT1340
		TE (TREAS.ED.I) ON TO 210	DAT1350
		IF (INLUGELWEI) OU TO EAN DEAN DEAN DEAN	DAT1340
		17 (14MADEC401) KEAD (K997V) DELA Do 200 (91) DIMM	DAT1270
		DE EUU UTITUMW	DATIZAN
		IF IIIANANCAI/ GU IU IV	DATISON
		リビビス (J) ギリビビス (J) ギア みし 1	DMIT220

		00 C 07 00	DAT1400
	190		DAT1410
	200		DATIAIU
	200		UA11420
	210		DAT1430
	210	REAU (2113) DELA	DAT1440
	220	IF (IRECU-EG.1) WHITE (2V13) DELX	DAT1450
		IF (IVAR.EQ.1.OR.IRECS.EQ.1.AND.IPRN.NE.1) WRITE (P.550) DELX	DAT1460
		IF (IVAR.NE.1.AND.IRECS.NE.1) WRITE (P.450) FACT	DAT1470
		READ (R,490) FACT, IVAR, IPRN, IRECS, IRECD	DAT1480
		IF (IRECS.EQ.1) GO TO 250	DAT1490
		IF (IVAR.EG.1) READ (R.490) DELY	DAT1500
		DO 240 I=1.DIML	DAT1510
		IF (IVAR.NE.1) GO TO 230	DAT1520
		DE(Y(I)) = DE(Y(I)) = FACT	DAT1530
		GQ TQ 240	DAT1540
	230		DAT1550
	240		DAT1560
	240		DAT1570
	350		DAT1580
	250	ΠΕΛΟ (2'14) ΟΕΙ' Τε (ΤΟΕΛΝ ΕΛ 1) ΜΟΤΤΕ (2114) ΝΕΙΥ	DATISON
	200	IF (INCODEW)] WHITE (2'14) DEC AND IDDA AF IN WRITE (D 560) DE Y	DAT1590
		IF (IVAN EQUIDUR INCOSEQUIDAND IFNN NELI) WRITE (P9380) DELI	DATIOUU
_		IF (IVARONEOIGANDOIRECSONEOI) WHILE (POOD) FACT	DATIOIO
C			UA11620
С		INITIALIZE VARIABLES	DAT1630
			DAT1640
		INO1=DIML-1	DAT1650
		IF (LEAK.NE.CHK(9).OR.SS.NE.0.) GO TO 280	DAT1660
		D0 270 I=2,IN01	DAT1670
		D0 270 J=2+JN01	DAT1680
		IF $(M(I,J),EQ,0,)$ GO TO 270	DAT1690
		TL (1+J)=RATE (1+J)/M(1+J)	DAT1700
	270	CONTINUE	DAT1710
	58 0	ETQB=0.0	DAT1720
		ETQD=0.0	DAT1730
		SUBS=0.0	DAT1740
		U=10	DAT1750
		TT=0.0	DAT1760
		IM=MIN0(6+DIMW+4+124)	DAT1770
		IM=(132-IM)/2	DAT1780
		VF4(3)=DIGIT(IM)	DAT1790
		VF4(8)=DIGIT(IM+5)	DAT1800
		WIDTH=0.	DAT1810
		D0 290 J=2+JN01	DAT1820
	290	WIDTH=WIDTH+DELX(J)	DAT1830
		YDIM=0.	DAT1840
		DO 300 I=2+INO1	DAT1850
	300	YDIN=YDIN+DELY(I)	DAT1860
	300	RETURN	DAT1870
c			DAT1880
č			DAT1890
č		READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD	-DAT1900
ř			DAT1910
č		FNTRY NEWPER	DAT1920
r			DAT1930
ř			DATIQAO
C		PEAD (PA490) KPAKPMIANHELATMAXANINTACHITADELT	DAT1950
r		NERV INTTON AFTACHINGGINANTANUNITUKII	DATION
~			DAT1970
L.		TT-DETT/24.	DATIORA
			DATIOGO
			DAT2000
		na nta t-tauali	3412000

			DAT2010
			0472020
			DAT2030
		IF (Im-GE, IMAX) GU IU 320	DAT2040
	310	CONTINUE	DATZOFO
		GO TO 330	DATEUSU
	320	DELT#TMAX/TM#DELT	DATZUOU
		NUMT=I	DA12070
	330	WRITE (P:570) KP:TMAX;NUMT;DELT:CDLT	DAT2080
		DELT=DELT#3600.	DAT2090
		TMAX=TMAX+86400.	DAT2100
С			DAT2110
ē		INITIALIZE SUMP. STRT. SL. WELL AND WR	DAT2120
-		WRITE (P.580) NWEL	DAT2130
		TE (KD.GT.KEMI) SUMPEO.	DAT2140
			DAT2150
			DAT2160
		UU 300 9-1101mm	DAT2170
			DAT2180
		STRI(1,))=PH(1,))	DAT2100
	340	IF (LEAK.NE.CHR(9)) 60 10 350	DA12170
		$IF (M(I \bullet J) \bullet EG \bullet 0 \bullet) GO TU 350$	DATZZVU
		SL(1+J)#RATE(1+J)/M(1+J)#(RIVER(1+J)=STRT(1+J))	DAT2210
	350	WELL(I,J)=0.	DAT2220
		IF (NW.EQ.0) GO TO 370	DAT2230
		DO 360 I=1,NW	DAT2240
	360	WR(I)=0.	DAT2250
	370	1F (NWEL-EG.0) GO TO 410	DAT2260
С			DAT2270
ř			DAT2280
Č			DAT2290
			DAT2300
			DAT2310
			DAT2320
		IF (RADIUS-EW-U-) GU 10 300	DAT2330
			0412330
		[F (KW-G1-NW) GO TO 380	DATESAU
		NWR (KW • 1) = I	UA12350
		NWR (KW+2)=J	UAIZJOU
		WR (KW) = RADIUS	DAT2370
		WRITE (P+590) I+J+WELL(I+J)+WR(KW)	DAT2380
		GO TO 390	DAT2390
	380	WRITE (P+590) I+J+WELL(I+J)	DAT2400
	390	WELL(I,J)=WELL(I,J)/(DELX(J)+DELY(I))	DAT2410
	400	CONTINUE	DAT2420
	A 10	RETURN	DAT2430
c	7		DAT2440
2			DAT2450
L A			DAT2460
C a		FURMATS	DAT2470
C			DATAGO
ç		***************************************	-UA12400
С			UA12490
С			UA12500
	420	FORMAT ('0',63X,'STARTING HEAD =',G15.7)	DAT2510
	430	FORMAT ('0'+41X+9A4+'='+G15.7)	DAT2520
	440	FORMAT (11++49X+9A4+/+65X+*MATRIX++/+50X+36(+-+))	DAT2530
	450	FORMAT ('0',72X,'DELX =',G15.7)	DAT2540
	460	FORMAT (101+72X+10FLY #1+615-7)	DAT2550
	470	FORMAT (1)1,60%, ISTARTING HEAD MATRIX1/61%-20(1-1))	DAT2560
	47V	FORMAT (11-4004) STANTING HEAD HAINTATATONALE (1-7)	DAT2570
	40V.	TURNAL (11144UA), CUNITIONATION - HEAD AFTER TURESTY, SEC FUMPING	DATORA
	400	[-7 + 2 + 7 + 3 + 7 + 7 + 7 + 7 + 7 + 7 + 7 + 7	DATOROA
	470	FURMAT (044040)	
	500	FURMAI (A440X,5010,0140)	DATZELA
	510	FURMAI (CVF4+U)	UNICOLU

	E20 FORMAT / MARSELY, INTERED OF RUNDING REPIONS
	SEV FORMAL (VVVSIATINGHER OF FOMFING FERIODS -VIS/474VVIME SIEFS BUAI2620
	IEIWEEN PRINTUUS = 137/314 PROPERTY CRIERIA FUR CLUSURE = 1613, 740416630
	21X V STEADY STATE ENROR CHITERIA = VG15.7/744X VSPECIFIC SDAT2640
	3TORAGE OF CONFINING BED =';G15.7/54X;'EVAPOTRANSPIRATION RATE =';GDAT2650
	415.7/56X, 'EFFECTIVE DEPTH OF ET ='.G15.7//22X, 'MULTIPLICATION FACTDAT266
	50R FOR TRANSMISSIVITY IN X DIRECTION = +,G15.7/63X, IN Y DIRECTION DAT2670
	6='•G15.7) DAT268(
	530 FORMAT (101+12+2X+20F6+1/(5X+20F6+1)) DAT269(
i	
	STO FORMAT (0) 1044 - ANNOTE CRACTER IN BOATATVEE IN A DIDECTION /ATV. ADDATOTO
	550 FORMAT (INTIGGAVGAVORID SPACING IN PROTOTIFE IN & DIRECTION/#/X#40DAT2/10
	560 FORMAT (IH-,46X,40HGRID SPACING IN PROTOTYPE IN Y DIRECTION/4/X,40DA12/30
	1('-')//('0'+12F10.0)) DAT2740
	570 FORMAT ('-'+50X+'PUMPING PERIOD N0.'+14+':'+F10.2+' DAYS'/51X+38('DAT2750
	1-1)//53X+ NUMBER OF TIME STEPS= + 16//59X+ DELT IN HOURS = ++F10.3//DAT2760
	253X • MULTIPLIER FOR DELT = • • F10 • 3) DAT2770
	580 FORMAT (1-1+63X+14+1 WELLS1/65X+9(1-1)//50X+11+9X+1) PUMPING RDAT2780
,	
	GIO FORMAL (YOY+30X) YON ALPHAMERIC MAPIY/40X, MULTIPLICATION FACTOR FODAT2820
	<pre>IR X DIMENSION =+,G15.7/40X, MULTIPLICATION FACTOR FOR Y DIMENSION DAT2830</pre>
	2=',G15.7/55X.'MAP SCALE IN UNITS OF ',A11/50X.'NUMBER OF ',A8,' PDAT2840
	3ER INCH =',G15.7/43X,'MULTIPLICATION FACTOR FOR DRAWDOWN =',G15.7/DAT2850
	447X. MULTIPLICATION FACTOR FOR HEAD = (.G15.7) DAT2860
	END DAT2870
	CHODANTINE CTED/DUT.KEED.CTDT.CHDT.T.WELL.DEDN.BATTAN.TAD.DELY.DDNSTP 1
	SUBMOUTINE STEPTENTYNEEFYSTNIYSGNIYTWEEEYERMYDDITUMYTUFYDEENYDDIST 20
	INITIALIZE DATA FOR TIME STEP, CHECK FOR STEADY STATE, STP 40
	PRINT AND PUNCH RESULTS SIP SU
0	sip 60
2	STP 70
2	SPECIFICATIONS: STP 80
	REAL #8PHI.DBLE.DABS.TEST2.DMAX1.XLABEL.YLABEL.XN1.MESUR.TITLE STP 90
	REAL #4MINS+M+KEEP STP 100
	INTEGER R.P.PU.DINI .DIMW.CHK.WATER.CONVRT.EVAP.CHCK.PNCH.NUM.HEAD.STP 110
	ICONTRAL FAK - FECH STD - ADT
^	
	DIMENSION $PHI(12, J2)$, $REEP(12, J2)$, $SIRT(12, J2)$, $SURI(12, J2)$, $I(12, SIP 140)$
	1JZ), BOTTOM(IP+JP), WELL(IZ+JZ), PERM(IP+JP), TOP(IC+JC), DELX(JZ)STP 150
	2, DDN(JZ), DELY(IZ), WR(IH), NWR(IH,2), ITTO(200), TEST3(IMX1) 5TP 160
C	STP 170
	COMMON /SARRAY/ VF4(11),CHK(15) STP 180
	COMMON ZSPARAMZ WATER.CONVRT.EVAP.CHCK.PNCH.NUM.HEAD.CONTR.EROR.IESTP 190
	1AK-BECH-SIP-U-SS-TT-THIN-FIDIST-OFT-FRP-THAX-COLT-HNAX-YDIM-WIDTH-STP 200
	2010 C - COD - ADT - DELT - CIMD - CIMD - CIDE - CTODE - TECT - ETOD - ETOD - EACTY - EACTY - CTD - 210
	ZNUMBILBUNIADIIUELIIJUMIBUMPIBUDBIBIUNEIIEBIIEIWDIEIWDIIAUININGINGIIJBI' CIV
	STERS MOUNT TERMAL MUMT ME MO NOTO MEN TEMAN LENGTH MUST ME STAL STORD 334
	3IERR .KOUNT . IF INAL .NUMT .KT .KP .NPER .KTH . ITMAX .LENGTH .NWEL .NW .DIML .DISTP 220
	3IERR • KOUNT • IF INAL • NUMT • KT • KP • NPER • KTH • ITMAX • LENGTH • NWEL • NW • DIML • DISTP 220 4MW • JNO1 • INO1 • R • P • PU • I • J • IDK1 • IDK2 STP 230
	3IERR+KOUNT+IFINAL+NUMT+KT+KP+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 COMMON /CK/ ETFLXT+STORT+GRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240
	3IERR+KOUNT+IFINAL+NUMT+KT+KF+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 COMMON /CK/ ETFLXT+STORT+GRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240 COMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IH+IMAX+IMX1 STP 250
	3IERR+KOUNT+IFINAL+NUMT+KT+KF+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 COMMON /CK/ ETFLXT+STORT+QRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240 COMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IH+IMAX+IMX1 STP 250 COMMON /PR/ XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP 260
	3IERR+KOUNT+IFINAL+NUMT+KT+KF+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 COMMON /CK/ ETFLXT+STORT+QRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240 COMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IH+IMAX+IMX1 STP 250 COMMON /PR/ XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP 260 1(60)+DIGIT(122)+VF1(6)+VF2(6)+VF3(7)+XSCALE+DINCH+SYM(17)+XN(100)+STP 270
	3IERR+KOUNT+IFINAL+NUMT+KT+KT+KP+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 STP 230 COMMON /CK/ ETFLXT+STORT+QRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240 COMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IH+IMAX,IMX1 STP 250 COMMON /PR/ XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP 260 1(60)+DIGIT(122)+VF1(6)+VF2(6)+VF3(7)+XSCALE+DINCH+SYM(17)+XN(100)+STP 270 2YN(13)+N1+N2+N3+YSCALE+FACT1+FACT2
~	3IERR+KOUNT+IFINAL+NUMT+KT+KF+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 COMMON /CK/ ETFLXT+STORT+GRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240 COMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IH+IMAX,IMX1 STP 250 COMMON /PR/ XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP 260 1(60)+DIGIT(122)+VF1(6)+VF2(6)+VF3(7)+XSCALE+DINCH+SYM(17)+XN(100)+STP 270 2YN(13)+NA(4)+N1+N2+N3+YSCALE+FACT1+FACT2 STP 280
с	3IERR+KOUNT+IFINAL+NUMT+KT+KF+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 COMMON /CK/ ETFLXT+STORT+GRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240 COMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+IS+JS+IH+IMAX,IMX1 STP 250 COMMON /PR/ XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP 260 1(60)+DIGIT(122)+VF1(6)+VF2(6)+VF3(7)+XSCALE+DINCH+SYM(17)+XN(100)+STP 270 2YN(13)+NA(4)+N1+N2+N3+YSCALE+FACT1+FACT2 STP 280 DATA DIE(3)+A1ED3(+VY)(700000000)
с	3IERR+KOUNT+IFINAL+NUMT+KT+KF+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP2204MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2STPCOMMON /CK/ ETFLXT+STORT+GRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNTSTPCOMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+IS+JS+IH+IMAX+IMX1STPCOMMON /PR/ XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP2601(60)+DIGIT(122)+VF1(6)+VF2(6)+VF3(7)+XSCALE+DINCH+SYM(17)+XN(100)+STP2702YN(13)+NA(4)+N1+N2+N3+YSCALE+FACT1+FACT2STPDATA PIE/3+141593/+YYY/Z00000000/STP
c	3IERR+KOUNT+IFINAL+NUMT+KT+KP+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP2204MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2STPCOMMON /CK/ ETFLXT+STORT+QRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNTSTPCOMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+IS+JS+IH+IMAX+IMX1STPCOMMON /PR/ XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP2601(60)+DIGIT(122)+VF1(6)+VF2(6)+VF3(7)+XSCALE+DINCH+SYM(17)+XN(100)+STP2702YN(13)+NA(4)+N1+N2+N3+YSCALE+FACT1+FACT2STPDATA PIE/3+141593/+YYY/Z00000000/STPRETURNSTP
c	3IERR+KOUNT+IFINAL+NUMT+KT+KP+NPER+KTH+ITMAX+LENGTH+NWEL+NW+DIML+DISTP 220 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 STP 230 COMMON /CK/ETFLXT+STORT+GRET+CHST+CHDT+FLUXT+PUMPT+CFLUXT+FLXNT STP 240 COMMON /ARSIZE/IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IH+IMAX+IMX1 STP 250 COMMON /PR/XLABEL(3)+YLABEL(6)+TITLE(5)+XN1+MESUR+PRNT(122)+BLANKSTP 260 1(60)+DIGIT(122)+VF1(6)+VF2(6)+VF3(7)+XSCALE+DINCH+SYM(17)+XN(100)+STP 270 2YN(13)+NA(4)+N1+N2+N3+YSCALE+FACT1+FACT2 STP 280 DATA PIE/3+141593/+YYY/Z000000000/ STP 310 RETURN STP 320



C		START A NEW TIME STEP	STP	340
C		*****	STP	350
-		FNTRY NEWSTP	STP	360
c			STP	370
•			STP	380
			STP	390
			STP	400
			CTD	410
		DO 10 J=1+DIMM	316	410
	10	KEEP(I,J)=PHI(I,J)	STP	420
		DELT=CDLT+DELT	STP	430
		SUM=SUM+DELT	STP	440
		SUMP=SUMP+DELT	STP	450
		DAYSP=\$UMP/86400.	STP	460
		YRSP=DAYSP/365.	STP	470
			STP	480
			STP	490
			CTP	500
		YRS=DAYS/365.	STP	510
		RETURN	STP	520
С			STP	530
Ċ			STP	540
Ċ		CHECK FOR STEADY STATE	STP	550
č		****************	STP	560
C		ENTRY STEARY	STP	570
~			STP	580
L.			CTD	500
			511	570
			515	600
		DO 50 7=5+2NO1	SIP	610
	20	TEST2=DMAX1(TEST2+DABS(DBLE(KEEP(I+J))-PHI(I+J)))	STP	620
		IF (TEST2.GE.EROR) GO TO 30	STP	630
		WRITE (P+330) KT	STP	640
		IFINAL=1	STP	650
		GO TO 40	STP	660
	30	TE (KT.EQ.NUMT) TETNALE)	STP	670
c	50		STP	680
ř			STP	690
č			CTP	700
Ľ			STP	710
С			SIF	710
		ENTRY TERMI	518	720
С			SIP	730
	40	IF (KT.GT.200) WRITE (P.400)	STP	740
		ITTO(KT)=KOUNT	STP	750
		IF (KOUNT.LE.ITMAX) GO TO 80	STP	760
		IERR=2	STP	770
		KOUNT=KOUNT=1	STP	780
			STP	790
			STP	800
~			CTP	210
C A		UNITE ON DIGHT OF DUNCH CADDO AS SPECIFIED IN THE OPTIONS,	CTD	010
ι			515	020
		XXX=SUM=DELT	515	830
		IF (IDK2.EQ.CHK(15)) WRITE (4) ((KEEP(I.J).YYY)I=1.DIML).J=1.DIMW)	STP	840
		1,XXX,SUMP,PUMPT,CFLUXT,GRET,CHST,CHDT,FLUXT,STORT,ETFLXT,FLXNT	STP	850
		IF (PNCH.NE.CHK(1)) GO TO 80	STP	860
		WRITE (PU+360) XXX+SUMP+PUMPT+CFLUXT+GRET+CHST+CHDT+FLUXT+STORT+ET	STP	870
		1FLXT+FLXNT	STP	880
		DO 50 I=1+DIML	STP	890
	50	WRITE (PU-350) (KEEP(T-J)-J=1-DIMW)	STP	900
	20		STP	910
	£ 0	UN IN UN. TE (TOMALED ANK(IELL MATTE (AL ANT.CHMLCHMA.DHART.AELHVT.ABET.ANET	STP	920
	οv.	IF (IUNE)EWAGHN(IJ)) WHILE (4) FHIJJUMJJUMFJFUMFIJUFLUAJAWHEIJUHJ A Gund Eliut etaut eliut eliut	CTD	030
		I SCHUI SFLOAI STURI SE IFLAI SFLANT	317	730
		IP (PNCM+NE+CHK(I)) GO TO BU	214	74 0

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WRITE (PU, 360) SUN, SUMP, PUMPT, CFLUXT, QRET, CHST, CHDT, FLUXT, STORT, ETSTP 950
     1FLXT+FLXNT
                                                                       STP 960
      DO 70 I=1,DIML
                                                                       STP 970
   70 WRITE (PU+350) (PHI(I+J)+J=1+DIMW)
                                                                       STP 980
                                                                       STP 990
С
   80 IF (CHCK.EQ.CHK(5)) CALL CHECK
                                                                       STP1000
      IF (IERR.EQ.2) GO TO 90
                                                                       STP1010
С
                                                                       STP1020
      ---PRINT OUTPUT AT DESIGNATED TIME STEPS---
C
                                                                       STP1030
      IF (MOD(KT,KTH).NE.0.AND.IFINAL.NE.1) RETURN
                                                                       STP1040
   90 WRITE (P+340) KT+DELT+SUM+MINS+HRS+DAYS+YRS+DAYSP+YRSP
                                                                      STP1050
      IF (CHCK.EQ.CHK(5)) CALL CWRITE
                                                                       STP1060
      IF (TT.NE.O.) WRITE (P.320) TMIN,TT
                                                                       STP1070
      KOUNT=KOUNT+1
                                                                       STP1080
      WRITE (P.300) (TEST3(J),J=1+KOUNT)
                                                                       STP1090
      WRITE (P+290) TEST2
                                                                       STP1100
      13=1
                                                                       STP1110
      15=0
                                                                       STP1120
  100 15=15+40
                                                                       STP1130
      I4=MIN0(KT,I5)
                                                                       STP1140
      WRITE (P+390) (I+I=I3+I4)
                                                                       STP1150
      WRITE (P+380)
                                                                       STP1160
      WRITE (P+370) (ITTO(I)+I=I3+I4)
                                                                       STP1170
      WRITE (P+380)
                                                                       STP1180
      IF (KT.LE.I5) GO TO 110
                                                                       STP1190
      I3=I3+40
                                                                       STP1200
      GO TO 100
                                                                       STP1210
C
                                                                       STP1220
     ---PRINT ALPHAMERIC MAPS---
                                                                       STP1230
C
  110 IF (CONTR.NE.CHK(3)) GO TO 120
                                                                       STP1240
      IF (FACT1.NE.O.) CALL PRNTA(1)
                                                                       STP1250
      IF (FACT2.NE.0.) CALL PRNTA(2)
                                                                       STP1260
  120 IF (HEAD.NE.CHK(8)) GO TO 140
                                                                       STP1270
                                                                       STP1280
С
      ---PRINT HEAD MATRIX---
                                                                       STP1290
C
      WRITE (P+310)
                                                                       STP1300
      DO 130 I=1+CIML
                                                                       STP1310
  130 WRITE (P+VF4) I+(PHI(I+J)+J=1+DIMW)
                                                                       STP1320
  140 IF (NUM.NE.CHK(4)) GO TO 170
                                                                       STP1330
С
                                                                       STP1340
С
      ---PRINT DRAWDOWN---
                                                                       STP1350
      WRITE (P+280)
                                                                       STP1360
                                                                       STP1370
C
      **************
     ENTRY DRDN
                                                                       STP1380
      ***
C
                                                                       STP1390
     DO 160 I=1,DIML
                                                                       STP1400
                                                                       STP1410
     DO 150 J=1,DIMW
  150 DDN(J)=SURI(I+J)=PHI(I+J)
                                                                       STP1420
 160 WRITE (P+VF4) I+(DDN(J)+J=1+DIMW)
                                                                       STP1430
 170 IF (NW.EQ.0.0R.IERR.EQ.1) GO TO 230
                                                                       STP1440
      С
С
                                                                       STP1460
     ---COMPUTE APPROXIMATE HEAD FOR PUMPING WELLS---
                                                                       STP1470
С
     WRITE (P+260)
                                                                       STP1480
                                                                       STP1490
     DO 220 KW=1+NW
     IF (WR(KW).EQ.0.) GO TO 220
                                                                       STP1500
      I=NWR(KW+1)
                                                                       STP1510
                                                                       STP1520
      J=N \models R(K = 2)
С
                                                                       STP1530
     COMPUTE EFFECTIVE RADIUS OF WELL IN MODEL---
                                                                       STP1540
C
      RE=(DELX(J)+DELY(I))/9.62
                                                                       STP1550
```

		5701540
	IF (WATER-NE-CHR(2)) GO TO 180	51F15C0
	IF (CONVRIANE.CHR(7)) GO TO 190	5121570
	IF (PHI(I,J).LT.TOP(I,J)) GO TO 190	STP1580
С	C	STP1590
С	CCOMPUTATION FOR WELL IN ARTESIAN AQUIFER	STP1600
	180 HW=PHI(I,J)+WELL(I,J)+ALOG(RE/WR(KW))/(2.+PIE+T(I,J))	<pre>#DELX(J)#DELYSTP1610</pre>
	1(1)	STP1620
		STP1630
~	60 10 210	STP1640
č	U - COMPLITATION FOD WELL IN WATER TARLE ANHTER	STP1450
C		STRIGUU
	140 HED=PHI(1+3)-BOILOW(1+3)	21P1600
	ARG=HED+HED+WELL(I+J)+ALOG(RE/WR(KW))/(PIE+PERM(I+J))	*DELX(J) *DELYSIPI6/0
	1(I)	STP1680
	IF (ARG.GT.0.) GO TO 200	STP1690
	WRITE (P,270) I,J	STP1700
	GO TO 220	STP1710
	$200 \text{ HW} = \text{SQRT}(\text{ARG}) + \text{EQTTOM}(1 \cdot \text{J})$	STP1720
c		STP1730
ř		STP1740
C		STP1750
		ST01760
	WHITE (FICOU) INGONK(KHITENHOKAW	5101770
	220 CONTINUE	STP1770
	230 IF (IERH.NE.2) RETURN	STP1780
	STOP	STP1790
С	C	STP1800
С	CDISK OUTPUT	STP1810
C .	C	STP1820
	ENTRY DISK	STP1830
С	C *****	STP1840
-	WRITE (4) PHI,SUM,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,FL	UXT.STORT.ETFSTP1850
	11 XT • FL XNT	STP1860
	DETURN	STP1870
c		STP1880
ĉ		STP1890
č		STRIGOO
č		5121900
C		STP1910
	ENTHT PUNCH	STP1920
С		5121930
	WRITE (PU,360) SUN,SUMP,PUMPT,CFLUXT,QRET,CHST,CHDT,F	LUXT, STORT, ETSTP1940
	1FLXT+FLXNT	STP1950
	DC 240 I=1.DIML	STP1960
	240 WRITE (PU+350) (PHI(I+J)+J=1+DIMW)	STP1970
	RETURN	STP1980
С	c	STP1990
ċ	-	STP2000
ž	-	6782010
C		51FEV10
С	C FORMATS:	SIFZUZU
С	C	STP2030
¢	C	STP2040
С	C	STP2050
С	C	STP2060
С	C	STP2070
	250 FORMAT (* *+43X+215+3F11+2)	STP2080
	260 FORMAT ('-'.50X. 'HEAD AND DRAWDOWN IN PUMPING WELLS'/	'51X+34('-')//STP2090
	148X+11 J WELL RADIUS HEAD DRAWDOWN1//)	STP2100
	270 FORMAT (1 1.43X.215.1 WELL IS DRY!)	STP2110
	280 FORMAT (1H1+60X+10RAWDOWN1/61X+8(1+1))	STP2120
	290 FORMAT (INMAXIMIM CHANGE IN HEAD FOR THIS TIME STEP	1.F10.3/1 1.551P2130
	13/1e11)	STP2140
	101-1777 200 EODMAT / FOMAYTMUM HEAD CHANGE EOD EACH TTEDATTONIFF/F	+.30(1_1)/(+ASTP215A
	JUU FURMAI ('UMAAIMUM NEAU UNANGE FUR EAUG IIERAHIUN'''' 11.16519.411	STP2160
	9.27AL92641	5,7 2200

310 FORMAT (*1*+60X+*HEAD MATRIX*/61X+11(*-*)) STP2170 320 FORMAT ('ODIMENSIONLESS TIME FOR THIS STEP RANGES FROM'+615,7, TSTP2180 10 · . G15 . 7) STP2190 STP2200 340 FORMAT (1H1+44X+57(+++)/45X++++14X++TIME STEP NUMBER =++19+14X+++STP2210 1+/45X,57(+-+)//50X,29HSIZE OF TIME STEP IN SECONDS=,F14.2//55X,+TOSTP2220 2TAL SIMULATION TINE IN SECONDS='+F14.2/80X+8HMINUTES=,F14.2/82X+6HSTP2230 3HOURS=+F14+2/83X+5HDAYS=+F14+2/82X+'YEARS=++F14+2///45X++DURATION STP2240 40F CURRENT PUMPING PERIOD IN DAYS=++F14.2/82X++YEARS=++F14.2//) STP2250 350 FORMAT (8F10.4) STP2260 360 FORMAT (4620.10) STP2270 370 FORMAT ('OITERATIONS: ++4013) STP2280 380 FORMAT (* *+10(*-*)) STP2290 390 FORMAT ('OTIME STEP 1',4013) STP2300 400 FORMAT (10+10(1++), THE NUMBER OF TIME STEPS EXCEEDS THE DIMENSIOSTP2310 IN OF THE VECTOR ITTO AND MAY CAUSE UNEXPECTED RESULTS IN ADDITIONASTP2320 2L1//OCOMPUTATION. AVOID PROBLEMS BY INCREASING THE DIMENSION OF TSTP2330 3HE VECTOR ITTO IN STEP++10(+++)) STP2340 END STP2350-SUBROUTINE SOLVE1 (PHI.BE.G.TEMP.KEEP.PHE.STRT.T.S.QRE.WELL.TL.SL.DSIP 10 1EL, ETA, V, XI, DELX, BET, DELY, ALF, TEST3, TR, TC, GRND, SY, TOP, RATE, M, RIVERSIP 20 SIP 2) 30 C 40 С SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE SIP 50 С 60 С SIP 70 С SPECIFICATIONS SIP 80 REAL #8PHI,DBLE,RHOP(20),G,BE,TEMP,DABS,W,TEST2,DMAX1,RHO,B,D+F,H,SIP 90 **SIP 100** 181 • E • CH • GH • BH • DH • EH • FH • HH • AL FA • BETA • GAMA • RES REAL #4KEEP+M SIP 110 INTEGER R,P,PU,DIML,DIMW,CHK,WATER,CONVRT,EVAP,CHCK,PNCH,NUM,HEAD,SIP 120 1CONTR.LEAK.RECH.SIP.IORDER(21).ADI **SIP 130 SIP 140** Ĉ DIMENSION PHI(1), BE(1), G(1), TEMP(1), KEEP(1), PHE(1), STRT(1), SIP 150 17(1), S(1), GRE(1), WELL(1), TL(1), SL(1), DEL(1), ETA(1), V(1), XSIP 160 2I(1), DELX(1), BET(1), DELY(1), ALF(1), TEST3(1), TR(1), TC(1), GRSIP 170 3ND(1), SY(1), TOP(1), RATE(1), M(1), RIVER(1) SIP 180 SIP 190 С COMMON /SARRAY/ VF4(11), CHK(15) SIP 200 COMMON /SPARAM/ WATER, CONVRT, EVAP, CHCK, PNCH, NUM, HEAD, CONTR, EROR, LESIP 210 1AK+RECH+SIP+U+SS+TT+TNIN+ETDIST+GET+ERR+TMAX+CDLT+HMAX+YDIM+WIDTH+SIP 220 2NUMS+LSOR+ADI+DELT+SUM+SUMP+SUBS+STORE+TEST+ETQB+ETQD+FACTX+FACTY+SIP 230 3IERR.KOUNT, IFINAL.NUMT, KT, KP, NPER.KTH, ITMAX, LENGTH, NWEL, NW, DIML, DISIP 240 SIP 250 4MW+JN01+IN01+R+P+PU+I+J+IDK1+IDK2 RETURN SIP 260 С SIP 280 Ć ---COMPUTE AND PRINT ITERATION PARAMETERS---SIP 290 С ***** **SIP 300** С SIP 310 ENTRY ITER1 ***** SIP 320 С ---INITIALIZE ORDER OF ITERATION PARAMETERS (OR REPLACE WITH A **SIP 330** C SIP 340 С READ STATEMENT) ----DATA IORDER/1,2,3,4,5,1,2,3,4,5,11*1/ SIP 350 SIP 360 12=IN01-1 J2=JN01=1 SIP 370 L2=LENGTH/2 **SIP 380** PL2=L2-1. SIP 390 SIP 400 W=0.

		PI≖0.	SIP 410
С			SIP 420
C		COMPUTE AVERAGE MAXIMUM PARAMETER FOR PROBLEM	SIP 430
		DO 10 I=2.INO1	SIP 440
		D0 10 J=2+JN01	SIP 450
		N=I+DIML+(J-1)	SIP 400
		IF (T(N)•EQ•O•) GO TO 10	SIP 470
		PI=PI+1.	SIF 400
		DX=DELX(J)/WIDTH	STP 500
		DY=DELY(I)/YDIM	317 500
		W=W+1. ~AMIN1(2. *DX*DX/(1. +FACTY*DX*DX/(FACTX*DY*DY)).2.*DY*DY/(1	+SIP 510
		IFACTX#DY#DY/(FACTY#DX#DX))	51F 520
	10	CONTINUE	SIP 530
~			STP 550
C			STP 560
L		Diel	SIP 570
			SIP 580
			SIP 590
	20	TEMP(I)=1(1W)**(PJ/PL2)	SIP 600
С	-		SIP 610
с		ORDER SEQUENCE OF PARAMETERS	SIP 620
		DO 30 J=1.LENGTH	SIP 630
	30	RHOP(J)=TEMP(IORDER(J))	SIP 640
		WRITE (P+370) HMAX	SIP 650
		WRITE (P,380) LENGTH, (RHOP(J), J=1, LENGTH)	SIP 600
		RETURN	SIP 680
ç			SIP 600
C		THATTAL THE DATA FOR A NEW ITERATION	STP 700
C		INITALIZE DATA FOR A NEW ITERATION	STP 710
	40	TE /VOINT LE TENAVI GO TO 50	STP 720
		IF (NUNI-LE-IIMAA) OU IU DU WRITE (D-360)	SIP 730
		CALL TERM]	SIP 740
	50	IF (MOD(KOUNT+LENGTH)) 60+60+70	SIP 750
С		*****	SIP 760
-		ENTRY NEWITA	SIP 770
Ç		**************************************	SIP 780
	60	NTH=0	SIP 790
	70	NTH=NTH+1	SIP 800
		W=RHOP(NTH)	SIP 810
		TEST3(KOUNT+1)=0,	510 020
			SIP 840
		NTUIMLYUIMW	STP 850
		DAC11-DAL11	STP 860
		PRE(1)=PRI(1) DEL(1)=0.	SIP 870
		FTA(1)=0.	SIP 880
		V(I)=0.	SIP 890
	80	XI(I)=0.	SIP 900
		BIGI=0.0	SIP 910
С			SIP 920
С		COMPUTE TRANSMISSIVITY AND T COEFFICIENTS IN WATER TABLE	51P 930
С		OR WATER TABLE-ARTESIAN SIMUATION	518 940 SIR 050
		IF (WATER.NE.CHK(Z)) GO TO 90	518 920 318 930
		CALL IMANS	STP 900
C			STP 980
L	90	TE (MOD (KOUNT.2)) 100.230.100	SIP 990
c	20		SIP1000
~		- ODER FOUNTIONS WITH DOW 1 FIRST - 212 FYANDIF!	STPIOIO
U.			

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C		123	SIP1020
č			SIP1030
ž			SIP1040
C	100	00 210 1=2, INO1	SIP1050
	100		5171000
			STP1070
		NL=N-DIML	STP1000
		NR=N+DIML	STP1100
			STP1110
		NB=N+1	STP1120
С			SIP1130
С		SKIP COMPUTATIONS IF NODE IS OUTSIDE AQUIFER BOUNDARY	SIP1140
		IF (T(N).EQ.0OR.S(N).LT.0.) GO TO 210	SIP1150
С			SIP1160
С		COMPUTE_COEFFICIENTS	SIP1170
		D=TR(NL)/DELX(J)	SIP1180
			SIP1190
		BETC(NA)/DELY(I)	SIP1200
		HEIC(N)/DELY(I)	SIP1210
c		IF (EVAFONE CHR(D)) GU IU IZU	SIP1220
ř			SIP1230
ç		FTORED.	SIF1240
			STP1250
		IF (PHE(N) LE.GRND(N) -ETDIST) GQ TQ 120	SIP1270
		IF (PHE(N).GT.GRND(N)) GO TO 110	STP1280
		ETQB=QET/ETDIST	SIP1290
		ETQD=ETGB+(ETDIST-GRND(N))	SIP1300
		GO TO 120	SIP1310
	110	ETQD=QET	SIP1320
С			SIP1330
C		COMPUTE STORAGE TERM	SIP1340
	120	IF (CUNVRI-EW-CHK(7)) GO TO 130	SIP1350
		RAUTS(N//DEL)	SIP1360
			S1P13/0
ſ			SIF1300
č		COMPUTE STORAGE COEFFICIENT FOR CONVERSION PROBLEM	STP1400
-	130	SUBS=0.0	STP1410
		IF (KEEP(N).GE.TOP(N).AND.PHE(N).GE.TOP(N)) GO TO 170	SIP1420
		IF (KEEP(N).LT.TOP(N).AND.PHE(N).LT.TOP(N)) GO TO 160	SIP1430
		IF (KEEP(N)-PHE(N)) 140,150,150	SIP1440
	140	SUBS=(SY(N)-S(N))/DELT*(KEEP(N)-TOP(N))	SIP1450
		GO TO 170	SIP1460
	150	SUBS=(S(N)-SY(N))/DELT*(KEEP(N)-TOP(N))	SIP1470
	160	RHOEST(N)/DELT	SIP1480
	170		SIP1490
	170		SIPISUO
~	180	IF (LEAK.NE.CHR(9)) GO TO 200	SIP1510
Ċ			SIP1520
Ľ		TE (RATE(N) FO.O. OR.N(N) FO.O.) GO TO 200	51715JU
		HED1=AMAX1 (STRT(N) + TOP(N))	3151240 3151240
		U=1.	STP1560
		HED2=0.	STP1570
		IF (PHE(N).GE.TOP(N)) GO TO 190	SIP1580
		HED2=TOP(N)	SIP1590
		U=0.	SIP1600
	190	SL (N) = RATE (N) / M(N) * (RIVER (N) - HED1) + TL (N) * (HED1-HED2-STRT (N))	SIP1610
	200	CONTINUE	SIP1620

~			CTD1630
C			STP1630
L			51F1040
С		PORWARD SUBSTITUTE, COMPUTING INTERMEDIATE VECTOR V	51F1050
			SIF1000
		CH=DEL (NA) +B/(1.+W+DEL (NA))	5191670
		GH=ETA(NL) #D/(1.+W#ETA(NL))	S1P1680
		BH=B=W*CH	SIP1690
		DH=D=W*GH	SIPI700
		EH=E+W*CH+W*GH	SIP1710
		FH=F-W*CH	SIP1720
		HH=H=W*GH	SIP1730
		ALFA=BH	SIP1740
		BETA=DH	SIP1750
		GAMA=EH-ALFA*ETA(NA)-BETA*DEL(NL)	SIP1760
		DEL (N) =FH/GAMA	SIP1770
		ETA (N) =HH/GAMA	SIP1780
		RES==D*PHI(NL)=F*PHI(NR)=H*PHI(NB)=B*PHI(NA)=E*PHI(N)=RHO*KEEP(N)=	SIP1790
	1	lsl(n)=gre(n)=well(n)+etqd=subs=tl(n)+strt(n)	SIP1800
		V (N) = (HMAX+RES-ALFA+V (NA) -BETA+V (NL))/GAMA	SIP1810
	210	CONTINUE	SIP1820
С			SIP1830
С		BACK SUBSTITUTE FOR VECTOR XI	SIP1840
		DO 220 I=1,12	SIP1850
		I3=DIML-I	SIP1860
		D0 220 J=1,J2	SIP1870
		J==DIM₩−J	SIP1880
		N=I3+DIML*(J3-1)	SIP1890
		IF (T(N).EQ.0OR.S(N).LT.0.) GO TO 220	SIP1900
		XI (N) =V (N) -DEL (N) +XI (N+DIML) -ETA (N) +XI (N+1)	SIP1910
С			SIP1920
С		COMPARE MAGNITUDE OF CHANGE WITH CLOSURE CRITERION	SIP1930
		TCHK=ABS(XI(N))	SIP1940
		IF (TCHK.GT.BIGI) BIGI=TCHK	SIP1950
		PHI(N)=PHI(N)+XI(N)	SIP1960
	220	CONTINUE	SIP1970
		IF (BIGI.GT.ERR) TEST=1.	SIP1980
		TEST3(KOUNT+1)=BIGI	SIP1990
		IF (TEST.EQ.1.) GC TO 40	SIP2000
		RETURN	S1P2010
¢			SIP2020
С			SIP2030
С		ORDER EQUATIONS WITH THE LAST ROW FIRST - 3X3 EXAMPLE:	SIP2040
Ċ		789	SIP2050
С		4 5 6	SIP2060
¢		123	SIP2070
С			SIP2080
	230	DO 340 II=1+I2	SIP2090
	-	I=DIML-II	SIP2100
		D0 340 J=2+JN01	SIP2110
		N=I+DIML*(J-1)	SIP2120
		NL=N-DINL	SIP2130
		NR=N+DIML	SIP2140
		NA=N-1	SIP2150
		NB=N+1	SIP2160
С			SIP2170
С		SKIP COMPUTATIONS IF NODE IS OUTSIDE AQUIFER BOUNDARY	SIP2180
		IF (T(N).EQ.0OR.S(N).LT.0.) GO TO 340	SIP2190
С			SIP2200
С		COMPUTE COEFFICIENTS	SIP2210
		D=TR(NL)/DELX(J)	SIP2220
		F=TR(N)/DELX(J)	51P2230

		B=TC(NA)/DELY(I)	SIP2240
		H=TC(N)/DELY(I)	SIP2250
		IF (EVAP.NE.CHK(6)) GO TO 250	S1P2260
C			SIP2270
С		COMPUTE EXPLICIT AND IMPLICIT PARTS OF ET RATE	SIP2280
		ETQB=0.	SIP2290
		ETQD=0.0	SIP2300
		IF (PHE(N).LE.GRND(N)-ETDIST) GO TO 250	STP2310
		IF (PHE(N).GT.GRND(N)) GO TO 240	STP2320
		ETQB=QET/ETDIST	STP2330
		ETQD=ETQB+(ETDIST-GRND(N))	STP2340
		GO TO 250	STP2350
	240	ETQD=QET	STP2360
С			STP2370
С		COMPUTE STORAGE TERM	SIP2380
	250	IF (CONVRT.EG.CHK(7)) GO TO 260	STP2390
	-	RHO=S(N)/DELT	SIP2400
		IF (WATER.EQ.CHK(2)) RHO=SY(N)/DELT	STP2410
		GO TO 330	SIP2420
С			STP2430
С		COMPUTE STORAGE COEFFICIENT FOR CONVERSION PROBLEM	STP2440
	260	SUBS=0.0	STP2450
	-	IF (KEEP(N).GE.TOP(N).AND.PHE(N).GE.TOP(N)) GO TO 300	STP2460
		IF (KEEP(N).LT.TOP(N).AND.PHE(N).LT.TOP(N)) GO TO 290	STP2470
		IF (KEEP(N)-PHE(N)) 270,280,280	STP2480
	270	SUBS = (SY(N) - S(N)) / DELT + (KEEP(N) - TOP(N))	STP2490
	-	GO TO 300	STP2500
	280	SUBSE(S(N) = SY(N)) / DEL TO (KEEP (N) = TOP (N))	6193510
	290		5172510
	2		5172520
	300	RHORS (N) ZDELT	5172530
	310	TE ((FAK-NE-CHK(9)) GO TO 330	5182540
С	5.0		5172550
č		COMPUTE NET LEAKAGE TERM FOR CONVERSION SIMULATION	5172500
•		IF (RATE(N) FR-0 R-M(N) FR-0-) GO TO 330	5172570
		HED] = AMAX1 (STRT(N) + TOP(N))	STP2500
			STP2600
			SIP2610
		IF (PHE(N).GE.TOP(N)) GO TO 320	STP2620
		HED2=TOP(N)	STP2630
		U=0.	STP2640
	320	SL(N) = RATE(N)/M(N) + (RIVER(N) = HED1) + TL(N) + (HED1 = HED2 = STRT(N))	S1P2650
	330	CONTINUE	STP2660
С			STP2670
С		SIP 'REVERSE' ALGORITHM	STP2680
č		FORWARD SUBSTITUTE. COMPUTING INTERMEDIATE VECTOR V	5192690
-		E=-B-D-F-H-RHO-TL(N)+U-ETQB	STP2700
		CH=DEL(NB)+H/(1.++W+DEL(NB))	SIP2710
		$GH = ETA(NL) + D/(1_0 + W + ETA(NL))$	STP2720
		BH=H-W*CH	STP2730
		DH=D~W@GH	STP2740
		EH=E+W*CH+W*GH	STP2750
		FH=F-W*CH	SIP2760
		HH=B=W+GH	SIP2770
		ALFA=BH	SIP2780
		BETA=DH	S1P2790
		GAMA=EH-ALFA*ETA(NB)-BETA*DEL(NL)	SIP2800
		DEL (N) =FH/GAMA	SIP2810
		ETA (N) =HH/GAMA	SIP2820
		RES=-D*PHI(NL)-F*PHI(NR)-H*PHI(NB)-B*PHI(NA)-E*PHI(N)-RHO*KEEP(N)-SIP2830
	1	LSL (N) =QRE (N) =WELL (N) +ETQD=SUBS=TL (N) +STRT (N)	S1P2840

	340) = (T 1	; (- ' N -	IMA	X+	RE	:S-	- A I	LFI	A #	۷ (NB) -	BE	Ŧ,	4	V (NL))	/(SAI	4 4													SI	P2	85	50 50
~	340	CON			10																															51	P2	87	20
č				-					• •••	TE	E	^ p		e c	+0	0	.	•																		31	02	0	30
C			204	1 U P : A	* 3		. 1 . 1	1 I 1 N I I	10	15	r	UR	v	E C	10	R.	^	1 -																		- 3 1 - C 1	02	00	20
		00	30	. ^	13			, NI 3	λŢ																											21	02	01	0
		00	33	. M /		111		•																												51	r c Da	001	10
		0.34	2.	. m #					• •																											31	101	01	20
		N=1	.3*	.01	. M L.) – 1	1)	. .	~ /			-	•		~	~	• •	-																51		.76	20
		10.0		(r	• • •	E. 4) • (/ • •		л • : •	5(N)	• L		0.	'.						,														21			50
		XT (N 7	# \	(1)		-01	. Ļ	(N) # /	× †		+U	Ιm	L,	- 6	. 17		N)	•,		1.14	=1	,												21			+0
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C			ÇC	1996	44	۲ <u>۲</u>	4 M	GT	AT.	I VI	JE	0	r	CH	AN	GE		1	1 M		,,,(121	UH	C.	CR	11	<u> </u>	101	N = 1							51	. 72	.96	20
		ICH	IK =	AL	55(. (.	1))		- -	~ •	+	~	~																					21			
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		IF	(E	310	51 •	GI	• 6	(RF	?)	T	ξS	ŢΞ	1.																							SI	183	.0.	10
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		RET	UF	١N																																51	(P.)	104	40
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	370	FOR	MA	ΔT.	(*	- (• • 4	54)	K 9	15	OL	UT	10	N	8 Y		FHI	E	ST	RC)N(3L.	Y	IM	PL	IC	11	P	RO	CE	DU	RE		4	5X1	15	(P3	1	40
		143(<u>'</u> -	.!)	•/	1	61	. X -	• • •	9E.	TA	= 1	۶F	5.	2)			. .				• -	_		_							_				51	12		50
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		END)																																	3.	1-3	1	10-
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	:	SNUM	IS -		SOR		101	[•]	DE	ĹŤ	• Ś	UM	• 5	ÚM	Ρ.	SI	JR	s.	ŝt	OF	RE	۰Ť۱	ES	τ.	ET	QB	, E	TQ	D•	FA	сŤ	x	FA	۲Ċ.	TY	, 50	R	2	3 O E
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		4 M W .	JN	101	. 1	N		R	• P	, P	U.	I.	Ĵ.	ID	K1	1	D	K2											_							S	R	2	50
		RET	'UF	RN														-																		S	R	20	60

с			.SOR	270
Ċ			SOR	280
С		WRITE ACCELERATION PARAMETER	SOR	290
С		*************	SOR	300
		ENTRY ITER2	SOR	310
С			SOR	320
		WRITE (P+490) Hotte (D-500) HNAY (ENGTH	SUR	330
		NRIIL (F9DUU) MMAA9LENGIN Detnon	SUR	350
~		RETURN	SOR	360
č		•••••	SOR	370
č		INITIALIZE DATA FOR A NEW ITERATION	SOR	380
Ũ	10	KOUNT=KOUNT+1	SOR	390
	• •	IF (KOUNT.LE.ITMAX) GO TO 20	SOR	400
		WRITE (P+510)	SOR	410
		CALL TERM1	SOR	420
C		华市公司专业会会委会会会会会会会会会会会会会会会会会会会会会会会会会会会会会会会会会会	SOR	430
		ENTRY NEWITB	SOR	440
С	~ ~		SOR	450
	20	TEST3(KOUNT+1)=0.	SOP	400
			SOR	480
		Nadimeruime	SOR	490
	30	Def (1) = Def (1)	SOR	500
	50		SOR	510
c		B1G1-V+V	SOR	520
ř			SOR	530
č		OR WATER TABLE-ARTESIAN SIMUATION	SOR	540
_		IF (WATER.NE.CHK(2)) GO TO 40	SOR	550
		CALL TRANS	SOR	560
С			.SOR	570
С			SOR	580
Ç		SOLUTION BY LSOR	SOR	590
С			SOR	600
	40		SUR	610
			SOR	630
			SOR	640
			SOR	650
			SOR	660
		NB=N+1	SOR	670
		NL=N-DIML	SOR	680
		NR=N+DIML	SOR	690
		BE(J)=0.0	SOR	700
		G (J) ≠0 •0	SOR	710
C			SOR	720
С		SKIP COMPUTATIONS IF NODE IS OUTSIDE AQUIFER BOUNDARY	SOR	730
•		IF (T{N}+EQ+0++OF+S{N}+LT+0+) GO TO 150	SOR	740
c			SUR	750
C			SOR	770
			SOR	780
		B = TC(N-1) / DELY(I)	SOR	790
		H=TC(N)/DELY(I)	SOR	800
		IF (EVAP.NE.CHK(6)) GO TO 60	SOR	810
С			SOR	820
С		COMPUTE EXPLICIT AND IMPLICIT PARTS OF ET RATE	SOR	830
			SOR	840
		CIUDEVOU TE (DHE(N) LE CONO(N)_ÉTOTET) CO TO 40	20K	020
		IF (PHE(N).GT.GPND(N) CO TO 50	SUR	870
		TE TENETATADIADUMATANA AA IA DA	304	010

		FTOR=OFT/FTD1ST	SOR	880
			SOR	890
			SOP	000
			SOR	910
_	50		SOR	020
С			SOR	720
С		COMPUTE STORAGE TERM	50K	930
	60	IF (CONVRT.EQ.CHK(7)) GO TO 70	SUR	940
		RHO=S(N)/DELT	SOR	950
		IF (WATER.EQ.CHK(2)) RHO=SY(N)/DELT	SOR	960
		GO TO 140	SOR	970
С			SOR	980
Č		COMPUTE STORAGE COEFFICIENT FOR CONVERSION PROBLEM	SOR	990
-	70	SUBS=0.0	SOR	1000
	•••	TE (MEED (N) GE TOP (N) AND PHE (N) GE TOP (N)) GO TO 110	SORI	010
		$ \mathbf{F} = \{\mathbf{x} \in [\mathbf{r}], \mathbf{r} \in [$	SOR	020
		$ \mathbf{I} = \{ \mathbf{N} \in [\mathbf{N} \setminus \mathbf{N} \mid \mathbf{N} \in [\mathbf{N} \setminus \mathbf{N} \mid \mathbf{N} \in \mathbf{N} \mid \mathbf{N} \in \mathbf{N} \mid \mathbf{N} \in \mathbf{N} \mid \mathbf{N} \in \mathbf{N} \} $	COPI	1030
	~ ~	IF (KEEF(N) = FHE(N)) OUFJUFJU = TOO(N)	CODI	1040
	80	SUBSE(ST(N)=S(N))/DELT*(REEP(N)=TOP(N))	COR	
		60 10 110	3041	1050
	90	SUBS=(S(N)-SY(N))/DELT*(KEEP(N)+TOP(N))	SUR	1000
	100	RHO=SY(N)/DELT	SOR	1070
		GO TO 120	SOR	1080
	110	RHO=S(N)/DELT	SOR	1090
	120	IF (LEAK.NE.CHK(9)) GO TO 140	SOR	1100
С			SOR	1110
С		COMPUTE NET LEAKAGE TERM FOR CONVERSION SIMULATION	SOR	1120
-		IF (RATE(N)_EG_0OR_M(N)_EG_0_) GO TO 140	SOR	1130
		HEDI=AMAX1 (STRT (N) + TOP (N))	SOR	1140
			SOR	1150
			SOR	1160
		$\frac{1}{16} \left(\frac{1}{16} + \frac{1}{16} \right) = \frac{1}{16} \left(\frac{1}{16} + \frac{1}{16} \right)$	SOR	1170
			SOP	1180
			SOP	1190
	120		SOR	200
	130	3L (N) 4KATE (N//M(N/*(KIVER(N/*REDI/*TE(N)*(REDI*REDE*STRT(N/)	E A D	1210
_	140	CONTINUE	30R	1210
C			SUR:	1220
С		FORWARD SUBSTITUTE, COMPUTING INTERMEDIATE VECTOR G	SUR.	1230
		E==D=F=B=H=RHO=TL (N) *U=ETQB	SUR	1240
		W=E=D+8E(J=1)	SUR.	1230
		BE(J)=F/W	SOR	1260
		_Q==B*PHI(NA)=H*PHI(NB)=RHO*KEEP(N)=SL(N)=QRE(N)=WELL(N)+ETQD=SUBS=	SOR	1270
	3	LTL (N) *STRT (N) =D*PHI (NL) =F*PHI (NR) =E*PHI (N)	SOR	1280
		G(J)=(Q=D+G(J=1))/W	SOR	1290
	150	CONTINUE	SOR	1300
С			SOR	1310
С		BACK SUBSTITUTE FOR TEMP	SOR	1320
-		D0 160 KN04=1+N03	SOR	1330
			SOR	1340
		TEMP (NOA) =6 (NOA) =8E (NOA) #TEMP (NOA+1)	SOR	1350
	160		SOR	1360
r	100	CONTINCE	SOR	1370
č			SOR	1380
Ŷ			SOR	1 1 9 0
			SOP	1400
		011/01-011/01-01/01/01/01/01/01/01/01/01/01/01/01/01/0	SOP	1410
_		PTI(N)=PTI(N)+TTAAX*(ETT(V)	SUN.	1420
C			507.	1420
С			SUR.	1430
		TCHK=DABS(TEMP(J))	SUR	1440
	_	IF (TCHK.GT.BIGI) BIGI=TCHK	SOR	1450
	170	CONTINUE	SUR	1460
		IF (BIGI.GT.ERR) TEST=1.	SOR	1470
		TEST3(KOUNT+1)=BIGI	SOR	1480

		IF (KOUNT.EQ.0) GO TO 10 IF (TEST.EQ.0.) RETURN	SOR1490 SOR1500
~			S081510
č			SOR1520
C			5001530
		IF (MOD(RUDNI) LENGTH) NE.U) GU IU	5081530
		GO TO 200	SUR1540
	180	D0 190 I=2,IN01	SUR1550
		D0 190 J≖2,JN01	SOR1560
		N=I+DIML#(J=1)	SOR1570
		IF (T(N).EQ.0.) GO TO 190	SOR1580
		PHI(N)=PHI(N)+ALFA(I)+BETA(J)	SOR1590
	190	CONTINUE	SOR1600
		GO TO 10	SCR1610
С			SOR1620
С			SOR1630
С		TWO DIMENSIONAL CORRECTION TO LSOR	SOR1640
č			SOR1650
ē			SOR1660
č			SOR1670
•	200	DO 210 I=1.DIM	SOR1680
	2		SOR1690
			SOR1700
	210		S0R1710
	610		S0R1720
			S081730
			S0R1740
			SOR1750
			SOR1760
_		4+U+	SOR1770
C		SUMMATION OF COFFETENTS FOR FACH ROW	SOR1780
C			SOP1790
			5081800
		N#I+DIML"(J=I)	50R1800
			SURIGIU
		NB=N+1	SURIDZU
		NL=N-DIML	SORIBJU
		NR=N+DIML	SOR1840
		IF (S(N).LT.0.) GO TO 330	SOR1850
		IF (T(N).EQ.0.) GC TO 320	SOR1860
С			SOR1870
С		COMPUTE COEFFICIENTS	SOR1880
		D=TR(N=DIML)/DELX(J)	SOR1890
		F≖TR (N) /DELX (J)	SOR1900
		B=TC(N-1)/DELY(I)	SOR1910
		H=TC(N)/DELY(I)	SOR1920
		TE (EVAP-NE-CHK(6)) GO TO 230	SOR1930
С			SOR1940
č		COMPUTE EXPLICIT AND IMPLICIT PARTS OF ET RATE	SOR1950
•		ETQ8=0.	SOR1960
		FTQD=0.0	SOR1970
		IF (PHF(N) = F, GRND(N) = FTDIST) GO TO 230	SOR1980
		IF (PHF (N) - GT - GEND (N)) GO TO 220	SOR1990
		FIGEOFICETOIST	S0R2000
			S082010
		EIND=EIND=(EIDISI=GRND(N))	S0R2020
			S082030
~	550		S082040
C			SORZOFO
С			500203040
	230	DUGE (LUNVRISEUSCHR(7)) GU IO 240	50R2000
			SUPECIO
		IF (WAIEK₀EĞ₀CHK(Z)) HHU≖SY(N)/DELI	CODONGA
		GO TO 310	3046030

	240	SUBS=0.0	SOR2100
	240	IF (KEEP(N), GE, TOP(N), AND, PHE(N), GE, TOP(N)) GO TO 280	50R2110
		IF (KEEP(N) LT. TOP(N) AND PHE(N) LT. TOP(N)) GO TO 270	50R2120
		IF (KEEP(N)-PHE(N)) 250+260+260	SOR2130
	250	$\sum_{n=1}^{\infty} (SP(n) - S(n)) / DET + (KEEP(n) - TOP(n))$	SOR2140
	230		SOR2150
	260	SUBSE (S(N) - SY(N)) / DELT# (KEEP(N) - TOP(N))	SOR2160
	270		S0R2170
	210		S0R2180
	280		S0R2190
	200	TE (FAK-NE-CHK(9)) GO TO 310	S0R2200
r	270	IF (LEAR ONE OCHA (9)) OU TO 310	S0R2210
č			SOR2220
C		F (RATE(N) = FQ_{-0} (R = $N(N)$ = FQ_{-0}) GO TO 310	SOR2230
			50R2240
			SOR2250
			SOR2260
		TE (PHE(N), GE TOP(N)) GO TO 300	S0R2270
			SOR2280
			S0R2290
	200	SI (N) = DATE (N) / M (N) + (RTVER (N) = HED1) + TI (N) + (HED1 = HED2 = STRT (N))	S0R2300
	310		S0R2310
~	310	CONTROL	S0R2320
C		A = A = B	S082330
		######################################	S0R2340
			S082350
			S082360
			5082370
		₩4.₩,₩5.1 (N) = FOA.CHOS.TI(N) *0 = FAI(NA) *0 = FAI(N) *00 = ALL(N) *0.5 (N) *0.	S082380
	220		S082390
~	320	CUNTINCE	S0R2400
c r			S0R2410
C			S082420
			S0R2430
		G(1)=(0-A#@(1-1))/W	S0R2440
	220		S0R2450
r	330	CONTROL	S0R2460
ř			SOR2470
C			SOR2480
		DO 340 KN04#1=N03	SOR2490
		NQ4EDIM -KNQ4	SOR2500
	340	A = FA (NOA) = G (NOA) = BF (NOA) = A (NOA) = A (NOA)	C002510
r	340		S0R2520
ř			SOR2530
č			S0R2540
~			5082550
			5002550
			50R2500 6092570
	350		SOR2580
	3.10		50R2590
			SORZEDIO
		R	SOR2610
			S082620
		Q=0.	S0R2630
c		- **	SORZEAD
č			5082650
-		DO 460 I=2.INO)	S0R2660
		N#T+DIML#(J=1)	S0R2670
			SOR2680
		NRaNel	50R2690
		NL =N=D TML	S0R2700

```
NR=N+DIML
                                                                           SOR2710
                                                                           SOR2720
      IF (S(N).LT.0.) GO TO 470
     IF (T(N).EQ.0.) GO TO 460
                                                                           SOR2730
                                                                           S0R2740
      D=TR(N=DIML)/DELX(J)
                                                                           SOR2750
      F=TR(N)/DELX(J)
                                                                           S0R2760
     B=TC(N-1)/DELY(I)
                                                                           SOR2770
     H=TC(N)/DELY(I)
                                                                           SOR2780
      IF (EVAP.NE.CHK(6)) GO TO 370
С
                                                                          SOR2790
      ---COMPUTE EXPLICIT AND IMPLICIT PARTS OF ET RATE---
                                                                          SOR2800
С
                                                                          SOR2810
      ETGB=0.
                                                                          S0R2820
      ETQD=0.0
      IF (PHE(N).LE.GRND(N)-ETDIST) GO TO 370
                                                                           SOR2830
      IF (PHE(N).GT.GRND(N)) GO TO 360
                                                                          SOR2840
                                                                          SOR2850
      ETQB=QET/ETDIST
                                                                          SOR2860
      ETQD=ETQB+(ETDIST+GRND(N))
                                                                          SOR2870
      GO TO 370
  360 ETQD=QET
                                                                           SOR2880
С
                                                                           SOR2890
                                                                           SOR2900
      ---COMPUTE STORAGE TERM---
С
  370 IF (CONVRT.EQ.CHK(7)) GO TO 380
                                                                           SOR2910
                                                                           S0R2920
      RHO=S(N)/DELT
                                                                           SOR2930
      IF (WATER.EQ.CHK(2)) RHO=SY(N)/DELT
                                                                           S082940
      GO TO 450
                                                                           S0R2950
C
      ---COMPUTE STORAGE COEFFICIENT FOR CONVERSION PROBLEM---
                                                                           S0R2960
C
  380 SUBS=0.0
                                                                           SOR2970
                                                                           S0R2980
      IF (KEEP(N).GE.TOP(N).AND.PHE(N).GE.TOP(N)) GO TO 420
      IF (KEEP(N).LT.TOP(N).AND.PHE(N).LT.TOP(N)) GO TO 410
                                                                           SOR2990
                                                                           S0R3000
      IF (KEEP(N)-PHE(N)) 390+400+400
                                                                           S0R3010
  390 SUBS=(SY(N)-S(N))/DELT*(KEEP(N)-TOP(N))
                                                                           S0R3020
      GO TO 420
                                                                           SOR3030
  400 SUBS=(S(N)-SY(N))/DELT*(KEEP(N)-TOP(N))
                                                                           S0R3040
  410 RHO=SY(N)/DELT
                                                                           SOR3050
      GO TO 430
                                                                           S0R3060
  420 RH0=S(N)/DELT
                                                                           S0R3070
  430 IF (LEAK.NE.CHK(9)) GO TO 450
                                                                           S0R3080
C
С
      ---COMPUTE NET LEAKAGE TERM FOR CONVERSION SIMULATION---
                                                                           S0R3090
                                                                           S0R3100
      IF (RATE(N).EQ.0..OR.M(N).EQ.0.) GO TO 450
                                                                           SOR3110
      HED1=AMAX1(STRT(N),TOP(N))
                                                                           SOR3120
      U=1.
      HED2=0.
                                                                           S0R3130
                                                                           SOR3140
      IF (PHE(N) GE TOP(N)) GO TO 440
                                                                           $083150
      HED2=TOP(N)
                                                                           SCR3160
      U=0.
  440 SL(N)=RATE(N)/M(N)*(RIVER(N)-HED1)+TL(N)*(HED1-HED2-STRT(N))
                                                                           SOR3170
                                                                           SOR3180
  450 CONTINUE
                                                                           SOR3190
С
                                                                           S0R3200
      A=A-D
      B1=D+F+RHO+TL(N)+U+ETQB
                                                                           S0R3210
                                                                           S0R3220
      B2=82+81
                                                                           SOR3230
      C=C=F
      Q=Q+(D*PHI(NL)+F*PHI(NR)+B*PHI(NA)+H*PHI(NB)+RHO*KEEP(N)+SL(N)+GRESOR3240
     1(N)+WELL(N)-ETQD+SUBS+TL(N)*STRT(N)+(B+H+B1)*PHI(N))
                                                                           S0R3250
                                                                           SCR3260
  460 CONTINUE
                                                                           SOR3270
С
                                                                           S0R3280
      ---COMPUTATION OF INTERMEDIATE VECTOR G---
С
      W=82-A+8E(J-1)
                                                                           SOR3290
                                                                           SOR3300
      BE(J)=C/W
      G(J) = (Q-A+G(J-1))/W
                                                                           SOR3310
```

	470		SUB	222	0
	470	CONTINGE	500	225	
С			50R	333	
C		BACK SUBSTITUTE FOR BETA	SOR	334	• 0
		NO3=DIMW-2	SOR	335	0
		DO 480 KN04=1.NO3	SOR	336	0
			SOR	337	0
		NG4-DINW-NNG4 B544 (NG6)-6(NG6)-85(NG6)88574(NG6+1)	SOP	338	ň
	40V		600	220	
		GO TO 180	SUR	339	0
С			SOR	340	0
С			SOR	341	. 0
r		FORMATS	SOR	342	20
č			SOR	343	10
č			SOP	344	
C			600	345	
С			SUR	343	10
С			SOR	340	10
	490	FORMAT ('-'+45X+'SOLUTION BY LINE SUCCESSIVE OVERRELAXATION'/46X+4	SOR	347	0
	1	12(+ +))	SOR	348	10
	500	FORMAT (1-1-26X-1ACCELERATION PARAMETER =1-F6-3-1 TWO DIMENSIONAL	SOR	349	0
	300	(1) CODECTION EVEN (15.1) TEDATIONS()	SOR	350	0
		I CURRECTION EVENT (13) THERMITONS'/	500	350	
	510	FURMAT ("VEACEEDED PERMITTED NUMBER OF ITERATIONS" + 39(""))	306	321	10
		END	SOR	352	20-
		SUBROUTINE SOLVE3(PHI.BE.G.TEMP.KEEP.PHE.STRT.T.S.QRE.WELL.TL.SL.	DADI	1	10
	1	IEL STANVAXI DEL X-BETA-DEL X-ALFA-XII - TESTA-TR-TC-GRND-SY-TOP-RATE-N	ADT	7	20
			ADT	-	30
_	4	C + KIVER)	AUI		10
С			AUI	-	10
С		SOLUTION BY THE ALTERNATING DIRECTION IMPLICIT PROCEDURE	ADI	5	50
С			ADI	•	0
С			ADI	7	10
ř		SPECTETCATIONS (ADI	E	10
0		DEAL BRUTING F. BUOD (20)-G.RE.TEND.TMK.DARS.W.PARAM.TEST2.DMAX1.01	ADI	Ś	0
		REAL SCHIYDDLLYNOF (2079) LYTCH YINYDADGYWH ANNYYCDTLYCHAATYD	ADT	10	10
	1		401		
		REAL #4KEEP,M	AUI	11	
		INTEGER R,P,PU,DIML,DIMW,CHK,WATER,CONVRT,EVAP,CHCK,PNCH,NUM,HEAD	ADI	14	:0
	1	1CONTR+LEAK+RECH+SIP+ADI	ADI	13	10
С			ADI	14	10
-		DIMENSION PHI(1), BF(1), G(1), TEMP(1), KEEP(1), PHE(1), STRT(1),	ADI	15	50
	•	T(1), S(1), OPE(1), WELL(1), TL(1), SL(1), DEL(1), FTA(1), V(1),	ADI	ī e	0
		$\frac{1}{1} \frac{1}{1} \frac{1}{2} \frac{1}{1} \frac{1}{2} \frac{1}$		17	20
	-	EI(1) = UELA(1) = UELA(1			
		3 TC(1), GRND(1), SY(1), TOP(1), RATE(1), M(1), RIVER(1)	AUI	10	10
С			AUI	13	10
		COMMON /SARRAY/ VF4(11),CHK(15)	ADI	- 20)0
		COMMON /SPARAM/ WATER, CONVRT, EVAP, CHCK, PNCH, NUM, HEAD, CONTR, EROR, LE	EADI	51	10
	•	1AK-RECH-STP-U-SS-TT-TMIN-FTDIST-GET-ERR-TMAX-CDLT-HMAX-YDIM-WIDTH	ADI	22	20
		ANIMS ALSOR AND ADDIT ASIMA SIMPASIAS ASTOREATEST AFTOR AFTON AFACTY AFACTY		23	10
	-	ENORGYLGUNYAUTYDELTYGUNYYGUNYYGUNYYUUTETTETTETUDYTAUTYTAUTYTAUTYTAUTYTAUTYTAUTYTAUTYT	TANT	21	in.
		3 IERR • KOUNT • IF INAL • NUMT • KT • KP • NPER • K IH • I IMAX • LENG IH • NWEL • NW • DIML • DI	LAUI	2	10
	4	4MW,JNO1,INO1,R,P,PU,I,J,IDK1,IDK2	AUI	23	20
		RETURN	ADI	26	0
C			AD I	- 27	10
ċ			ADI	- 28	30
ř			ADI	29	0
č			ADT	30	٥.
C			ADT	21	0
			401	31	30
С		***************************************	AUL	30	: 0
		HMIN=2.	ADI	33	10
		IN4=DIMw-2	ADI	- 34	+0
		IN5=DIML-2	ADI	35	50
		XVAL =3,1415442/(2,4IN4+IN4)	ADI	36	50
		YVAL =3.15.5462//2.6TN54TN51	ADT	37	10
		1786-3817137 "C/ 108 "113"1137		- 1	10
		no in testunt	-01	50	

Г

	DO 10 1-2, INO1	ADT	390
	DO TO GELONOI		400
	N#I+DIML*(J=1)	AUI	400
	TE (T(N)-EQ-0-) GO TO 10	ADI	410
		ADT	420
	APARI=AVAL*(1)(1+DELA(0)**2*FACT1)DEL1(1)**2*FACTA)		
	YPART=YVAL+(1/(1+DELY(I)++2+FACTX/DELX(J)++2+FACTY))	ADI	4 .10
	LIMINGANINI (LIMIN, YDADI, YDADI)	ADI	440
			450
10	CONTINUE	AUI	430
	AL PHA=FXP(A) OG(HMAX/HMIN)/(LFNGTH=1))	ADI	460
		ADT	470
	KHOA(1) # 4 M 1 V	AUT	410
	DO 20 NTIME=2.LENGTH	ADI	480
		AD T	490
20	RHUP(N)IME) = RHUP(N)IME = 1/ *ALFHA		F 0 0
	WRITE (P+400)	ADI	500
		ADT	510
	WRITE (P9410) LENGTH9(RHOP(J)9J#I+LENGTH)	AUL	510
	RETURN	ADI	520
~			530
C			EAN
С		AUI	340
C		ADI	550
·		ADT	560
30		MUL	300
	IF (KOUNT-LE-ITMAX) GO TO 40	ADI	570
	WRITE (P. 30A)	ADI	580
	WRITE (FJ370)	407	600
	CALL TERM1	AUI	240
۵ ۵	TE (MOD(KOUNTALENGTH)) 50.50.60	ADI	600
		ADT	610
Ç	**************		
	ENTRY NEWITC	ADI	620
~		ADI	630
L		ADT	660
50		AUI	040
60	NTHENTHAI	ADI	650
00		ADI	660
	TEST3(KQUNT+1)≡0.	ADI	670
		ADI	680
		401	400
	NEDIML#DIMW	AUI	070
	DQ 70 T=1-N	ADI	700
-		ADT	710
70	PHF(T)=but(T)	MU I	
	BIGI=0.0	AUI	120
~		ADI	730
C		ADT	740
	COMPUTE TRANSMISSIVITY AND I CUEFFICIENTS IN WATER TABLE	AUT	740
C			
C	OR WATER TARIE-ARTESIAN SINUATION	ADI	750
C C	OR WATER TABLE-ARTESIAN SIMUATION	ADI	750 760
C C	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80	ADI ADI	750 760
C C	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 Call Trans	ADI ADI ADI	750 760 770
c c	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS	ADI ADI ADI ADI	750 760 770 780
c c	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 Call Trans	ADI ADI ADI ADI ADI	750 760 770 780 780
c c c	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS	ADI ADI ADI ADI ADI	750 760 770 780 790
	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI	ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800
с с с с с с с с	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI	ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800 810
сс ссссс	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI	ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800 810
	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI COMPUTE IMPLICITLY ALONG ROWS	ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800 810 820
	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800 810 820 830
с с с с с с с е с а	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI COMPUTE IMPLICITLY ALONG ROWS NO3=DIMW-2 DO GO (=).DIMW	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800 810 820 830 840
C C C C C C C 80	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI COMPUTE IMPLICITLY ALONG ROWS NO3=DIMW-2 DO 90 J=1+DIMW	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800 810 820 830 830
C C C C C C C C C C C C C C C C C C C	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 790 800 810 820 830 840 850
	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI COMPUTE IMPLICITLY ALONG ROWS NO3=DIMW-2 DO 90 J=1+DIMW N=1+DIML*(J=1) TEMP(1)=PHI(N)	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 800 810 820 830 840 850 860
C C C C C C C 80 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI COMPUTE IMPLICITLY ALONG ROWS NO3=DIMW-2 DO 90 J=1.0IMW N=1+DIML*(J-1) TEMP(J)=PHI(N) PO 900 J=2 OV	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 800 810 820 830 830 840 850 840 850 840
C C C C C C C C C C C S C S S S S S S S	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 800 810 820 830 840 850 860 860
C C C C C C C C C C S O 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI compute implicitly along Rows NO3=DIMW-2 DO 90 J=1.DIMW N=1+DIML*(J=1) TEMP(J)=PHI(N) DO 230 I=2.DIML DO 200 J=2.JNO1	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 800 810 820 830 840 850 860 860 860 880
C C C C C C C C S 0 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI COMPUTE IMPLICITLY ALONG ROWS NO3=DIMW-2 DO 90 J=1.0IMW N=1+DIML*(J=1) TEMP(J)=PHI(N) DO 230 J=2.0IML DO 200 J=2.JNO1 N=TADIML*(J=1)	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 800 810 820 830 840 850 840 850 840 850 840 850 840 850 840 850 840 850 840 850 850 850 850 850 850 850 850 850 85
C C C C C C C C C C 80 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 800 810 820 830 840 850 840 850 860 880 880 880
C C C C C C C C C C S C S O S O	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 780 800 810 830 830 830 840 850 840 850 840 850 840 850 890 900
C C C C C C C C S O 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 790 810 820 830 840 850 840 850 860 870 880 890 910
C C C C C C C C C C 80 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 780 800 810 820 830 840 830 840 830 840 830 840 890 900 920
C C C C C C C C C S O 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 770 790 810 820 830 830 830 830 830 840 850 840 850 840 890 910 920
C C C C C C C C S O 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 780 800 820 820 820 820 840 850 840 850 840 850 840 8900 910 920 930
C C C C C C C C C C S 0 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 780 810 820 830 840 830 840 850 8900 910 910 920 920 940
C C C C C C C C C C S O 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 780 800 810 830 830 830 830 830 830 900 920 930 930 930 930 950
C C C C C C C S C S O 90	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 760 780 8820 8820 8820 8820 8820 8820 8820
C C C C C C C C C C C C C C C C C C C	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	$\begin{array}{c} 750\\ 760\\ 770\\ 800\\ 810\\ 820\\ 830\\ 840\\ 850\\ 840\\ 850\\ 8900\\ 910\\ 900\\ 910\\ 900\\ 940\\ 950\\ 950\\ 950\\ 960\\ \end{array}$
C C C C C C C C C C C C C C C C C C C	OR WATER TABLE=ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI COMPUTE IMPLICITLY ALONG ROWS NO3=DIMW-2 DO 90 J=1.DIMW N=1+DIML*(J=1) TEMP(J)=PHI(N) DO 230 J=2.DIML DO 200 J=2.JNO1 N=I+DIML*(J=1) NA=N-1 NB=N+1 NL=N-DIML NR=N+DIML BE(J)=0.0 G(J)=0.0 SKIP COMPUTATIONS IF NODE IS OUTSIDE AGUIFER BOUNDARY	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	$\begin{array}{c} 750\\ 760\\ 780\\ 810\\ 810\\ 830\\ 840\\ 850\\ 840\\ 850\\ 9910\\ 930\\ 9950\\ 970\\ 950\\ 970\\ 970\\ 970\\ 970\\ 970\\ 970\\ 970\\ 97$
С С С С С С С С С С С С С С С С С С С	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 7760 780 8820 8820 8820 8820 8820 8820 8820
C C C C C C C C C C C C C C C C C C C	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	750 770 780 8820 8820 8820 8820 8820 8820
С С С С С С С С С С С С С С С С С С С	OR WATER TABLE-ARTESIAN SIMUATION IF (WATER.NE.CHK(2)) GO TO 80 CALL TRANS SOLUTION BY ADI 	ADI ADI ADI ADI ADI ADI ADI ADI ADI ADI	$\begin{array}{c} 750\\ 760\\ 780\\ 810\\ 800\\ 830\\ 840\\ 850\\ 840\\ 850\\ 990\\ 930\\ 990\\ 990\\ 990\\ 990\\ 990\\ 99$

-			
C		COMPUTE COEFFICIENTS	ADI1000
		D=TR(N-DIML)/DELX(J)	ADI1010
		F=TR(N)/DELX(J)	ADI1020
		B=TC(N-1)/DELY(I)	ADI1030
		H=TC(N)/DELY(I)	ADI1040
		TE (EVAP-NE-CHK(6)) GO TO 110	ADT1050
c			ADT1050
ř			AD11000
č		ETOGA	AUIIU/U
			AUIIUGU
			AD11090
		$IF (PHE(N) \cdot LE \cdot GRND(N) - EIDISI) GO TO 110$	ADI1100
		IF (PHE(N).GT.GRND(N)) GO TO 100	ADI1110
		ETQ8=QET/ETDIST	ADI1120
		ETQD=ETQB*(ETDIST-GRND(N))	ADI1130
		GO TO 110	ADI1140
	100	ETQD=QET	ADI1150
С			ADI1160
С		COMPUTE STORAGE TERM	AD11170
	110	IF (CONVRT.EQ.CHK(7)) GO TO 120	AD11180
	• • •	RH0=S(N)/DFLT	ADT1190
		TE (WATER-EG.CHK(2)) RHOESY(N)/DELT	ADT1200
			ADT1210
~			ADIIZIO
ř			AUIIZZU
U	120	SUBSEA A	AU11230
	120		AD11240
		IF (REEP(N).GE.IUP(N).AND.PHE(N).GE.TOP(N)) GO TO 160	AD11250
		IF (REEP(N).LT.TOP(N).AND.PHE(N).LT.TOP(N)) GO TO 150	ADI1260
		IF (REEP(N) - PHE(N)) 130, 140, 140	ADI1270
	130	SUBS=(SY(N)-S(N))/DELT*(KEEP(N)-TOP(N))	ADI1280
		GO TO 160	ADI1290
	140	SUBS=(S(N)-SY(N))/DELT*(KEEP(N)-TOP(N))	ADI1300
	150	RHO=SY(N)/DELT	ADI1310
		GO TO 170	ADI1320
	160	RHO=S(N)/DELT	ADI1330
	170	IF (LEAK.NE.CHK(9)) GO TO 190	ADI1340
С			ADI1350
С		COMPUTE NET LEAKAGE TERM FOR CONVERSION SIMULATION	4011360
		IF (RATE(N).EQ.0OR.M(N).EQ.0.) GO TO 190	ADI1370
		HEDI=AMAXI (STRT (N) + TOP (N))	ADT1380
			ADT1300
			AD11390
		TE (DHE(N)) CE TOB(N)) CO TO 100	AD11400
			AUII4IU
			A011420
			ADI1430
	190	SL(N) #HATE(N)/M(N) * (RIVER(N) = HED1) + TL(N) * (HED1=HED2=STRT'(N))	ADI1440
_	190	CONTINUE	ADI1450
С			ADI1460
С		CALCULATE VALUES FOR PARAMETERS USED IN THOMAS ALGORITHM	ADI1470
C		AND FORWARD SUBSTITUTE TO COMPUTE INTERMEDIATE VECTOR G	ADI1480
		IMK=(B+D+F+H)+PARAM	ADI1490
		E=-D-F-RHO-IMK-TL (N) +U-ETQB	ADI1500
		₩=E-D+8E(J-1)	AD11510
		BE(J)=F/W	AD11520
		Q=-8*PHI (NA) + (8+H-IMK-E) *PHI (N) -H*PHI (NR) -RHO*KEEP (N) -SI (N) -OPE (N)	ADT1530
	1	L-WELL (N) + ETGD-SUBS-TL (N) + STRT (N) + D+PHT (N) + F+PHT (NP)	ADT1540
	•	$G(J) = \{(q=D+G(J=1)) / W$	ADT1550
	200	CONTINUE	ADT1644
С			ADT1500
č			AD11594
-		XII (DIMU)30-DO	MU11200
			MOT12A0
		DO EEU NOUTTINUS	A011000

			ADT1610
			ADI1610
r			AU11620
č			AU11630
C			AU11040
		$\frac{1}{16} \frac{1}{100} \frac{1}{100} = 100000000000000000000000000000000$	AUI1030
			AU11660
			AUIIOTU
	- • •	GO 10 220	ADI1680
	210	XII(NU4)=G(NU4)=GE(NO4)=XII(NO4+1)	ADI1690
	220	(EMP(N04)=PH1(N)+XII(N04)	ADI1700
-	S30	CONTINUE	ADI1710
C			ADI1720
С			ADI1730
¢		COMPUTE IMPLICITLY ALONG COLUMNS	ADI1740
		NO3=DIML-2	ADI1750
		DO 240 I=1.DIML	ADI1760
	240	TEMP(I)=PHI(I)	ADI1770
		DO 380 J=2,0CIMW	ADI1780
		DO 350 I=2,INO1	ADI1790
		N=I+DIML+(J-1)	ADI1800
		NA=N-1	ADI1810
		NB=N+1	ADI1820
		NL=N-DIML	ADI1830
		NR=N+DIML	ADI1840
		BE(I)=0.0	ADT1850
		G(T) = 0	ADT1860
C			ADT1870
č		SKIP COMPUTATIONS IF NODE IS OUTSIDE AQUITER ROUNDARY	ADT1880
•		$\mathbf{T} = \{\mathbf{T}(\mathbf{N}) \in \mathbf{F}_{0}, 0, 0, 0, 0, 1, 1, 0, 1, 0, 0, 1, 0, 3, 0, 0, 1, 0$	ADT1990
c			ADT1000
č		COMPUTE COFFEICIENTS	ADT1910
v			AUT1920
			AU11720
			AUI1930
			AU11940
		$\frac{1}{16} \frac{1}{16} \frac$	AU11950
~		IF (EVAP+NE+CHN(0)) 00 10 200	AD11900
C			AD11970
C		TOP-0	AD11980
			AD11990
			AD12000
		IF (PHE(N).LE.GRND(N)-ETDIST) GO TO 260	AD12010
		IF (PHE(N).GT.GRND(N)) GO TO 250	02021DA
		ETQB=QET/ETDIST	AD12030
		ETQD=ETQB*(ETDIST-GRND(N))	ADI2040
		GO TO 260	ADI2050
	250	FTQD=QFT	4012060
С	200		AD12070
ē		COMPUTE STORAGE TERM	4012080
Ŭ	260	1F (CONVRT_EG_CHK(7)) GO TO 270	AD12090
	200		AD12100
		TF (WATER-EQ-CHK(2)) RHO=SY(N)/DELT	AD12110
			AD12120
c		00 10 340	AD12120
ž			ADT2140
C			AD12140
	610	JUDJEVAU	AU12130
		IF $(R \in [n], 0] \in [UP(n], AND, PHE(N), 0] \in [UP(N]) = UU(N]$	ADIZIOU
		IF (NEEP(N) + LI + IUP(N) + AND + PHE(N) + LI + IUP(N)) GU IU 300	AU12170
		17 (REEP(N)=PHE(N)) 28092904290	AU12180
	200	303-131(N/-3(N))/UELI*(KEEP(N)-(OP(N))	AU12190
		60 10 310	AD15500
	290	SUB5={S(N)-SY(N))/DELT+(KEEP(N)~TOP(N))	AD12210
	300	RHO=SY(N)/DELT	AD15550
--------	-----	--	---------
		GO TO 320	ADI2230
	310	RHO=S(N)/DELT	AD12240
	320	TF (LEAK . NE . CHK (9)) GO TO 340	AD12250
c	52.		AD12260
ž			ADT2270
C		TE (DATE (N) SO A OB M(N) SO A CONVENSION SIMULATION	AD12290
		$[\mathbf{r} (RAie(\mathbf{N}) - EW + \mathbf{U}	AU12200
		HEDI=AMAXI(STRT(N)+TOP(N))	AU12290
		U=1.	AD12300
		HED2=0.	ADI2310
		IF (PHE(N).GE.TOP(N)) GO TO 330	ADI2320
		HED2=TOP(N)	AD12330
		U=0.	ADI2340
	330	SI (N)=RATE(N)/M(N)*(RIVER(N)=HED1)+TL(N)*(HED1=HED2=STRT(N))	AD12350
	340		AD12360
~	340	CONTINUE	AD12370
C A		AN AND ATT WALKED FOR RADAMETERS HEED IN THOMAS ALCORTTHM	ADIDIA
C		CALCULATE VALUES FUR PARAMETERS USED IN INUMAS ALGURITHM	AU12300
Ç		AND FORWARD SUBSTITUTE TO COMPUTE INTERMEDIATE VECTOR G===	AU12390
		IMK=(B+D+F+H)+PARAM	AD12400
		E=-8-H-RHO-IMK-TL(N)+U-ETQB	ADI2410
		W=E+B+BE(I+1)	ADI2420
		BE(I)=H/W	ADI2430
		<pre>g==D*PHI(NL)+(D+F=IMK+E)*PHI(N)=F*PHI(NR)=RHO*KEEP(N)=SL(N)=QRE(N)</pre>	ADI2440
	1	-WELL (N) + FTOD-SUBS-TL (N) + STRT (N) - B + PHI (NA) - H + PHI (NB)	AD12450
	•	G(T) = (R - R + G(T - 1)) / W	AD12460
	250		ADT2470
~	350	CONTINUE	AD12480
ž			ADT2400
C		STACK SUBSTITUTE FOR HEAD VALUES AND FLACE THEM IN TEMPOOR	A012470
		XII(DIML)=0.00	AU12500
		D0 370 KN04=1,N03	ADI2510
		NO4=DIML-KNO4	ADI2520
		N=N04+DIML+(J-1)	ADI2530
С			ADI2540
С		FIRST PLACE TEMP VALUES IN PHI(N=DIML)	ADI2550
		PHI(N-DIML)=TEMP(NO4)	ADI2560
		1F (T(N) NE.0.AND.S(N) GE.0.) GO TO 360	ADI2570
		XTT(NO4)=0.00	AD12580
			AD12590
			AD12600
	360		AD12610
	300		AD12610
~		IEHP(NO4/ + PHI(N) + XII(NO4))	AD12020
ç			AU12030
Ç			AU12640
		TCHK=ABS(SNGL(TEMP(NO4))-PHE(N))	AD12650
		IF (TCHK.GT.BIGI) BIGI=TCHK	ADI2660
	370	CONTINUE	ADI2670
	380	CONTINUE	ADI2680
		IF (BIGI.GT.ERR) TEST=1.	ADI2690
		TEST3(KOUNT+1)=BIGI	ADI2700
		IF (TEST.EQ.1.) GO TO 30	AD12710
		RETURN	AD12720
С			AD12730
ř			AD12740
č			AD12750
ž			1012760
č			AD12774
C C			AD12794
			AD12700
C			AD12130
	340	FURMAL (FUEALEEUEU FERMITIEU NUMBER OF ITERATIONS// 1,39(141))	AU12800
	400	FURMAI (T=T+38X, SOLUTION BY THE ALTERNATING DIRECTION IMPLICIT PR	AU12810
	1	LOCEDURE 1/39X + 56 (1_1)	AD12820

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	410 FORMAT (///1H0,15,22H ITERATION PARAMETERS:,8D12,3//28X,10D12,3) END	ADI: ADI:	2830 2840-
	SUBROUTINE COEF(PHI,KEEP,PHE,STRT,SURI,T,TR,TC,S,WELL,TL,SL,PERM,B	COF	10
	10TTOM,SY,RATE,RIVER,M,TOP,GRND,DELX,DELY)	COF	20
С		COF	30
C	COMPUTE COEFFICIENTS	COF	40
ç	***************************************	COF	50
C		COF	20
C		COF	80
	REAL "GENITUDELTENNOTS UTFIN	COF	90
	INTEGER R.P. PU. DIML. DIMW. CHK. WATER. CONVRT. EVAP. CHCK. PNCH. NUM. HEAD.	COF	100
	1CCNTR+LEAK+RECH+SIP+ADI	COF	110
С		COF	120
	DIMENSION PHI(1), KEEP(1), PHE(1), STRT(1), SURI(1), T(1), TR(1),	COF	130
	1TC(1), S(1), WELL(1), TL(1), SL(1), PERM(1), BOTTOM(1), SY(1), RAT	COF	140
_	2E(1), RIVER(1), M(1), TOP(1), GRND(1), DELX(1), DELY(1)	COF	150
С		COF	100
	COMMON /SARKAT/ VF4(II)+CHK(IS) Common /Sarkat/ VF4(II)+CHK(IS)	COF	180
	LOWMON JEARAMY WATERCONVECTORS AFTOR THE AND THE ADVICE TAMMAX AND TAMAX AND THE	COF	190
	2NUMS-L SOR+ADT-DELT-SUMP-SUMP-SUBS-STORF-TEST-FTQR-FTQD-FACTX-FACTY-	COF	200
	31ERR + KOUNT + IF INAL + NUMT + KT + KP + NPER + KTH + ITMAX + LENGTH + NWEL + NW + DIML + DI	COF	210
	4MW, JNC1, INC1, R, P, PU, I, J, IDK1, IDK2	COF	220
С		COF	530
	DATA PIE/3.141593/	COF	235
_	RETURN	COF	240
c		COF	250
C	CONDUCT COTESTATENTS FOR TRANSIENT BART OF LEAKAGE TERM	COF	200
c c	ARABABABABABABABABABABABA Arababababababababababababababababababab	COF	280
ç		COF	290
С	**************************************	COF	300
-	TMIN=1.E30	COF	310
	T T = 0 • 0	COF	320
	PFATE=0.	COF	330
	DC 50 I=1+DIML	COF	340
		COF	350
~	N = I + UIML + (J - I)	COF	300
с с		COF	380
c c	HEAD BOUNDARY	COF	390
C	IF (RATE(N).LE.0OR.T(N).EQ.0OR.M(N).EQ.0OR.S(N).LT.0.) GO TO	COF	400
	1 50	COF	410
С		COF	420
С	IF VALUE FOR TL(N) WILL EQUAL VALUE FOR PREVIOUS NODE.	COF	430
С	SKIP PART OF COMPUTATIONS	COF	440
	IF (RATE(N) + M(N), EQ. PRATE) GO TO 40	COF	450
	$\bigcup_{i=1}^{N} \bigcup_{j=1}^{N} \bigcup_{i=1}^{N} \bigcup_{j=1}^{N} \bigcup_{j=1}^{N} \bigcup_{i=1}^{N} \bigcup_{j=1}^{N} \bigcup_{j=1}^{N} \bigcup_{i=1}^{N} \bigcup_{j=1}^{N} \bigcup_{i=1}^{N} \bigcup_{j=1}^{N} \bigcup_{j$	COF	400
	IF (UIM);GI;FI, II=UIM] IF (DIMT,IT,TMIN) TMIN=DIMT	COF	480
		COF	490
С		COF	500
r	RECOMPUTE PPT IF DIMT WITHIN RANGE FOR SHORT TIME COMPUTATION	COF	510
Č	IF (DIMT+LT+1+0E=03) PPT=1+0/DIMT	COF	520
	CC=(2,3-PPT)/(2,*PPT)	COF	530
С		COF	540
С	COMPUTE SUM OF EXPONENTIALS	COF	550
	SUMN=0.0	COF	560

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		DO 20 K=1.200	COF	570
		POWER=K*K*PPT	COF	580
		IF (POWER+LE+150+) GO TO 10	COF	590
		POWER=150	COF	600
	10	PEX=EXP(+POWER)	COF	610
		SUMN=SUMN+PEX	COF	620
		$\frac{1}{10} \left(\frac{1}{10} + \frac{1}{10}$	COF	630
	~ ~		COF	640
~	20	CONTINUE	COP	650
č			COF	470
L	30	OFNOM-1.0	COF	490
	30	TE (DIMT_) T_1.0F=03) DENOM=SORT(DIE#DIMT)	COF	400
C			COF	700
č		HEAD VALUES ARE NOT INCLUDED IN COMPUTATION OF Q FACTOR SINCE	COF	710
č		LEAKAGE IS CONSIDERED IMPLICITLY	COF	720
-	40	Q1=RATE(N)/(M(N)+DENOM)	COF	730
	-	TL (N)=Q1+2.+Q1+SUMN	COF	740
		PRÀTE=RATE (N) *M (N)	COF	750
	50	CONTINUE	COF	760
		TMIN#TMIN#3.0	COF	770
		TT#TT#3.0	COF	780
		RETURN	COF	790
Ç			COF	800
ç			COF	810
C			COF	820
C			COP	810
c			COF	040
C			COF	020
			COF	870
			COF	880
		IF (PERM(N) = Eq. 0.) GO TO 60	COF	890
		HED=PHI(N)	COF	900
		IF (CONVRT.EQ.CHK(7)) HED=AMIN1(SNGL(PHI(N)),TOP(N))	COF	910
		T (N) = PERM (N) * (HED-BOTTOM (N))	COF	920
		IF (T(N),GT.0.) GO TO 60	COF	930
		IF (WELL(N)+LT+0+) GO TO 70	COF	940
С			COF	950
Ç		THE FOLLOWING STATEMENTS APPLY WHEN NODES (EXCEPT WELL NODES)	COF	960
С		GO DRY	COF	970
			COF	980
			COF	990
			LOFI	1000
			COFI	010
			COFI	020
			COPI	030
		PAI(N)=SURI(N)	COPI	040
	4 0	PONTALE (FY130/ 190	COFI	050
	90	CONTINUE TE (KT_FQ_0) DETURN	COFI	070
			COFI	080
с			COFI	090
C		START PROGRAM TERMINATION WHEN A WELL GOES DRY	COFI	100
	70	WRITE (P+120) I+J	COFI	110
		WRITE (P+130)	COFI	120
		IERR=1	COF1	130
		CALL DRDN	COF1	140
		DO 80 I=2, INO1	COF1	150
			COF 1	160
		N=I=DIME=(J=1)	C0F1	1/0

Г

	80	PHI (N) = KEEP (N)	COF	180
		SUM=SUM=DELT	COF	190
			COFI	200
		KIEKITI TE (MT EQ A) STAR	COPI	1210
		IF (NICEWOU) STUP	COFI	220
		IF (IDREDEWOCHR(ID)) CALL DISR	COFI	240
		IF (FNCH-LEG-CHR(I/) CALL FONCH	COFI	250
		IF (MODINISKIH) (EGGU) SIOF	COFI	260
			COFI	270
		TE (CHCK_EQ_CHK(5)) CALL CWRITE	COFI	280
		STOP	COF	290
c			COFI	300
č		COMPUTE T COEFFICIENTS	COF	310
č		************	COF	320
		ENTRY TCOF	COF	330
С		****	COF	340
	90	DO 110 I=1.IN01	COFI	350
		D0 110 J=1+JN01	COFI	360
		N=I+DIML ⁴ (J+1)	COFI	370
		NR=N+DIML	COFI	380
		NB≖N+1	COFI	390
		IF $(T(N) \cdot EQ \cdot O \cdot)$ GO TO 110	COFI	400
		IF (T(NR)+EG+0+) GO TO 100	COFI	410
		TR(N)=(2++T(NR)+T(N))/(T(N)+DELX(J+1)+T(NR)+DELX(J))+FACTX	COFI	420
	100	IF (T(NB) • EQ • 0 •) GO TO 110	COFI	430
		TC(N)=(2.*T(NB)*T(N))/(T(N)*DELY(I+1)+T(NB)*DELY(I))*FACTY	COFI	440
	110	CONTINUE	COF	450
_		RETURN	COP	460
C		5004430	COPI	470
C			COFI	480
C			COFI	1490 1500
C			COF .	1500
С			COP	1510
С			COP	1520
	150	FORMAT (1-************************************	COP	1530
	130	FORMAT ('1',50X,'DRAWDOWN WHEN WELL WENT DRY')	COP.	1540
	140	FORMAT (11, 32X, DRAWDOWN FOR TIME STEP, 13, 13 SIMULATION TIME = .	CUP .	1990
		11PE15.7, 'SECONDS')	COP	1300
	150	FORMAT (*=*,20(***),** NODE *,14,***,14,** GUES DRY *,20(***))	COF	15970
		END	CUP.	1900-
		SUBROUTINE CHECKI (PHI, KEEP, PHE, STRT, T, TR, TC, S, GHE, WELL, TL, PERM, BO	CHK	10
		ITOM, SY, HATE, HIVEN, M. TOP, GRNU, DELX, DELT)	CHR	20
C			CHA	30
C		COMPUTE A MASS BALANCE		4U 50
Č				<u>60</u>
č		SPECIFICATIONS,	CHK	70
C		RFAL #APHTADALF	CHK	80
		DEAL BAKEED.M	CHK	90
		INTEGER RAPADINDINI ADIMWACHKAWATERACONVRIAEVARACHCKARACHANUMAMEANA	CHK	100
	1	ICONTRAL FAKARFCHASIPAADI	CHK	110
c		* AAU 111 2 PRUIS 11 PUT 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	CHK	120
ç		DIMENSION PHI(12+JZ) + KEEP(12+JZ) + PHE(12+JZ) + STRT(12+JZ) + T(12+J	CHK	130
	1	12) • TR(1Z+JZ) • TC(1Z+JZ) • S(1Z+JZ) • GRE(1Z+JZ) • WELL(1Z+JZ) • TL(1Z	CHK	140
	2	2,JZ), PERM(IZ,JZ), BOTTOM(IP,JP), SY(IP,JP), RATE(IR,JR), RIVER(IR	CHK	150
	3	3,JR), M(IR,JR), TOP(IC,JC), GRND(IL,JL), DELX(JZ), DELY(IZ)	СНК	160
С			СНК	170
		COMMON /SARRAY/ VF4(11)+CHK(15)	снк	180

		COMMON /SPARAM/ WATER, CONVRT, EVAP, CHCK, PNCH, NUM, HEAD, CONTR, EROR, LIAK, RECH, SIP, U, SS, TT, TMIN, ETDIST, GET, ERR, TMAX, CDLT, HMAX, YDIM, WIDTH	ЕСНК • СНК	190 200
		2NUMS+LSOR+ADI+DELT+SUM+SUMP+SUBS+STORE+TEST+ETQB+ETQD+FACTX+FACTY	CHK	210
		3IERR,KOUNT, IF INAL,NUMT,KT,KP,NPER,KTH,ITMAX,LENGTH,NWEL,NW,DIML,D	I CHK	220
		4MW•JN01•IN01•R•P•PU•I•J•IDK1•IDK2	СНК	230
		COMMON /CK/ ETFLXT,STORT,QRET,CHST,CHDT,FLUXT,PUMPT,CFLUXT,FLXNT	СНК	240
		COMMON /ARSIZE/ IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IH+IMAX+IMX1	СНК	250
		RETURN	СНК	260
С		• • • • • • • • • • • • • • • • • • • •	CHK	270
С		\$\$\$\$\$ \$ \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$	СНК	280
		ENTRY CHECK	СНК	290
C		****	СНК	300
С		INITIALIZE VARIABLES	СНК	310
		PUMP=0.	СНК	320
		STOR=0.	Снк	330
		FLUXS=0.0	СНК	340
		CHD1=0.0	СНК	350
		CHD2≖0.0	СНК	360
		QREFLX=0.	СНК	370
		CFLUX=0.	СНК	380
		FLUX=0.	CHK	390
		ETFLUX=0.	CHK	400
~		FLXN=U.U	CHK	410
C		•••••••••••••••••••••••••••••••••••••••	CHK	420
č				430
C			CHK	450
			СНК	460
		IF (T(I+J)+FQ+0+) = GQ = TQ - 240	CHK	470
		AREA=DELX(J)+DELY(I)	CHK	480
		IF (S(I,J).GE.0.) GO TO 120	CHK	490
С			CHK	500
С		COMPUTE FLOW RATES TO AND FROM CONSTANT HEAD BOUNDARIES	СНК	510
		IF (S(I+J=1)+LT+0++0R+T(I+J=1)+EQ+0+) GC TO 30	СНК	520
		X=(STRT(I+J)~PHI(I+J-1))#TR(I+J-1)#DELY(I)	СНК	530
		IF (X) 10,30,20	СНК	540
	10	CHD1=CHD1+X	СНК	550
		GO TO 30	СНК	560
	20	CHD2=CHD2+X	CHK	570
	30	IF (S(I,J+I) + LT + 0 + 0R + 1(I,J+I) + EQ + 0 +) GO TO GO	СНК	580
		X= (S(R) (1+J)-PHI(1+J+I))*(R(1+J)*DELT(I)	CHK	590
	4.0		CHK	600
	40			010
	6 0		CHK	630
	٥ <u>ر</u>	$\frac{1}{16} \left(\frac{1}{16} \right) \left(\frac{1}{16} \right) \left(\frac{1}{16} \right) = \frac{1}{16} \left(\frac{1}{16} \right)	CHK	640
	00	17 (3(1-1)-0)-01(0-0)(1-1)-0)-0-00(0) (0 (0 (0 (0 (0 (0 (0 (0 (0 (0 (0 (0 (0	CHK	660
		TF (X) 70-90-80	CHK	660
	70		CHK	670
		GQ TO 90	CHK	680
	80	CHD2=CHD2+X	CHK	690
	90	IF (S(I+1+J)+LT+0++OR+T(I+1+J)+EQ+0+) GO TO 240	CHK	700
		X=(STRT(I+J)-PHI(I+1+J))*TC(I+J)*DELX(J)	СНК	710
		IF (X) 100+240+110	СНК	720
	100	CHD1=CHD1+X	СНК	730
		GO TO 240	СНК	740
	110	CHDZ=CHDZ+X	CHK	750
~		U IU 24U	CHK	/00
C C			CHK	7790
L	120	ORFFLXEORFFLXEORF(I) #ARFA	CHK	790
	100	AURI RUMAURI RULAURI 1114 A.	Grin	120

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IF (WELL(I+J)) 130+150+140
                                                                        CHK 800
                                                                       CHK 810
 130 PUMP=PUMP+WELL(I,J) *AREA
     GO TO 150
                                                                        CHK 820
 140 CFLUX=CFLUX+WELL(I,J)#AREA
                                                                        CHK 830
 150 IF (EVAP.NE.CHK(6)) GO TO 190
                                                                        CHK 840
                                                                       CHK 850
С
С
      ---COMPUTE ET RATE---
                                                                        CHK 860
      IF (PHI(I,J),GE,GRND(I,J)-ETDIST) GO TO 160
                                                                       CHK 870
                                                                       CHK 880
      ETQ=0.0
      GO TO 180
                                                                        CHK 890
  160 IF (PHI(I,J).LE.GRND(I,J)) GO TO 170
                                                                       CHK 900
      ETQ=QET
                                                                       CHK 910
                                                                        CHK 920
      GO TO 180
  170 ETQ=QET/ETDIST+(PHI(I,J)+ETDIST-GRND(I,J))
                                                                        CHK 930
  180 ETFLUX=ETFLUX-ETQ#AREA
                                                                       CHK 940
                                                                       CHK 950
C
      ---COMPUTE VOLUME FROM STORAGE---
                                                                        CHK 960
C
  190 STORE=S(I+J)
                                                                        CHK 970
      IF (WATER.EQ.CHK(2)) STORE=SY(I.J)
                                                                        CHK 980
                                                                       CHK 990
      IF (CONVRT.NE.CHK(7)) GO TO 230
      X = KEEP(I + J) + PHI(I + J)
                                                                       CHK1000
      IF (X) 200,210,210
                                                                        CHK1010
  200 HED1=PHI(I+J)
                                                                        CHK1020
                                                                        CHK1030
      HED2=KEEP(I+J)
      X=ABS(X)
                                                                        CHK1040
                                                                        CHK1050
      GO TO 220
  210 HED1=KEEP(I+J)
                                                                        CHK1060
      HED2=PHI(I,J)
                                                                        CHK1070
                                                                        CHK1080
  220 STORE=S(I+J)
      IF (HED1=TOP(I.J).LE.O.) STORE=SY(I.J)
                                                                        CHK1090
      IF ((HED1-TOP(I,J))*(HED2-TOP(I,J)).LT.0.0) STORE=(HED1-TOP(I,J))/CHK1100
     1X#S(I+J)+(TOP(I+J)-HED2)/X#SY(I+J)
                                                                        CHK1110
  230 STOR=STOR+STORE*(KEEP(I,J)-PHI(I,J))*AREA
                                                                        CHK1120
                                                                        CHK1130
С
С
      ---COMPUTE LEAKAGE RATE---
                                                                        CHK1140
      IF (LEAK.NE.CHK(9)) GO TO 240
                                                                        CHK1150
      IF (M(I+J).EQ.0.) GO TO 240
                                                                        CHK1160
                                                                        CHK1170
      HED1=STRT(I+J)
      IF (CONVRT.EQ.CHK(7)) HED1=AMAX1(STRT(I,J),TOP(I,J))
                                                                        CHK1180
                                                                        CHK1190
      HED2=PHI(I+J)
      IF (CONVRT.EQ.CHK(7)) HED2=AMAX1(SNGL(PHI(I+J))+TOP(I+J))
                                                                        CHK1200
      XX=RATE(I,J)*(RIVER(I,J)-HED1)*AREA/N(I,J)
                                                                        CHK1510
      YY=TL(I+J)*(HED1-HED2)*AREA
                                                                        CHK1220
      FLUX=FLUX+XX
                                                                        CHK1230
      XNET=XX+YY
                                                                        CHK1240
      FLUXS=FLUXS+XNET
                                                                        CHK1250
      IF (XNET.LT.0.) FLXN=FLXN=XNET
                                                                        CHK1260
  240 CONTINUE
                                                                        CHK1270
С
      CHK1290
C
С
      ---COMPUTE CUMULATIVE VOLUMES, TOTALS, AND DIFFERENCES---
                                                                        CHK1300
                                                                        CHK1310
      STORT=STORT+STOR
      STOR=STOR/DELT
                                                                        CHK1320
      ETFLXT=ETFLXT=ETFLUX*DELT
                                                                        CHK1330
                                                                        CHK1340
      FLUXT=FLUXT+FLUXS*DELT
                                                                        CHK1350
      FLXNT=FLXNT+FLXN#DELT
      FLXPT=FLUXT+FLXNT
                                                                        CHK1360
      QRET=QRET+QREFLX*DELT
                                                                        CHK1370
                                                                        CHK1380
      CHDT=CHDT-CHD1+DELT
      CHST=CHST+CHD2+DELT
                                                                        CHK1390
      PUMPT=PUMPT-PUMP*DELT
                                                                        CHK1400
```

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	CFLUXT=CFLUXT+CFLUX*DELT	CHKI	1410
	TOTI 1=STORT+QRFT+CFLUXT+CHST+FLXPT	CHK	1420
	TOTI 2=CHDT+PUMPT+FTFLXT+FLXNT	CHK	430
		CHK	1440
			1440
		CHK	1430
	PERCNT=0.0	CHK	1460
	IF (TOTL2.EQ.0.) GO TO 250	CHK	1470
	PERCNT=DIFF/TOTL2#100.	CHK	1480
	250 RETURN	CHK	1490
~		CHK	1500
Ļ			1200
С	c	CHK]	1510
c	C PRINT RESULTS	CHK	1520
ř		CHK	1530
C		CHK	1540
		CHK	1660
С		CHR	1540
С	C	CHN.	1000
	WRITE (P+260) STOR+QREFLX+STORT+CFLUX+QRET+PUMP+CFLUXT+	ETFLUX+CHSTCHK:	1570
	1,FLXPT,CHD2,TOTL1,CHD1,FLUX,FLUXS,ETFLXT,CHDT,SUMR,PUMP	T+FLXNT+TOTCHK	1580
	21.2 • DIFF • PERCNT	CHK	1590
	DETION	СНК	1600
~	RETURN A	СНК	1610
C	C	CHIC	1420
С	CFORMATS	CHN	1020
C	C	CHK.	1030
С	C	CHK	1640
С	c	СНК	1650
ř		CHK	1660
-	240 FORMAT (101-107-101010 ATTVE MASS BALANCE 1-161-164-11 ##31-2	3X. PATES FCHK	1670
	200 FURMAL (0 FILATION CAN LAND ALLAND ALLAND ALLAND A PLAN	1420Y - LEOUCHK	1490
	10R THIS TIME STEPTY 16X + L + 3/1 / /11X + 24 () + 3/1 23 ()		1000
	2RCES11,69X,1STORAGE =1,F20,4/20X,8(1-1),68X,1HECHARGE =	* • F 20 • 4/2/XUAN	1030
	3, ISTORAGE = I, F20, 2, 35X, ICONSTANT FLUX = I, F20, 4/26X, IREC	HARGE = + F2CHK	1700
	40.2.41X. PUMPING =	IOX . 'EVAPOTRCHK	1710
	54NSPTRATION =1+F20-4/21X+1CONSTANT HEAD =1+F20-2+34X+1C	ONSTANT HEACHK	1720
	60+1/277, 11 FAKAGE =1, 520, 2,467,110 =1,520,4/217,110 TAL 5	OURCES = . FCHK	1730
	DUITZIAT LEARAGE - TT EVELTUATIN - TT COTTERS	ASX JEROM CHK	1740
	/20.24 JAJ UUT = + F 20.47 JAJ LEANAGE - / 20.4 UISCHARGES	-1-520 4/10HK	1760
	APREVIOUS PUMPING PERIOD = ++P20+4/20x+II(+-+)+88x++101AL	= + + P 2 V • 4 / 1 CHK	1730
	96X+'EVAPOTRANSPIRATION ='+F20+2/21X+'CONSTANT HEAD ='+F	20.2,36X, SCHK	1760
	SUM OF RATES = ++F20.4/19X+QUANTITY PUMPED = ++F20.2/27X++	LEAKAGE = + + CHK	1770
	\$F20.2/19X. TOTAL DISCHARGE = 1. F20.2//17X. DISCHARGE-SOL	RCES = + F20CHK	1780
	5. 2/15Y + PEP CENT DIFFERENCE = 1. E20-2//)	СНК	1790
		СНК	1800-
	END	Citik	1000
	SUBROUTINE PRNTAT(PHI-SUBI-T-S-WELL-DELX-DELY)	PRN	10
~		PRN	20
C .			30
С	C PRINT MAPS OF DRAWDOWN AND HTDRAULIC READ	PRN	30
C	C	,PHN	40
С	c	PRN	50
ċ	C SPECIFICATIONS:	PRN	60
č	DEAL HONT 7-YI AREL YI AREL TITI FAXNI MESHR	PRN	70
	REAL BEFLYEVALADELY/LADELY/LEEYAATTHESOR	DDN	8 Ô
	REAL THE OLD ON OTHER OTHER OUR WATER COMPTENTS CHAR CHAR DI		60
	INTEGER ROPOPUODINLODINUOCHROWATEROCONVRIGEVAPOCHCROPNC	HINUMINEAUIPHN	70
	1CONTR+LEAK+RECH+SIP+ADI	PRN	100
С	C	PRN	110
	DIMENSION PHI(IZ,JZ), SURI(IZ,JZ), S(IZ,JZ), WELL(IZ,JZ)	<pre>!) + DELX(JZ)PRN</pre>	120
	1 • DELY(IZ) • T(IZ • JZ)	PRN	130
r		PRN	140
U	COMMON SCADEANS NEATING CHESTEN	DON	150
	CUMMUN /SARHAT/ VF4(11)+CHK(13)		160
	CUMMUN /SPAHAM/ WATER CUNVRISEVAPSCHCKSPNCHSNUMSHEADSC	IN IR PERURALEMAN	100
	1AK,RECH,SIP,U,SS,TT,TMIN,ETDIST,GET,ERR,TMAX,CDLT,HMAX,	TUIM, WIDTH, PRN	1/0
	2NUMS+LSOR+ADI+DELT+SUM+SUMP+SUBS+STORE+TEST+ETQB+ETQD+F	ACTX + FACTY + PRN	180
	3IERR+KOUNT+IFINAL+NUMT+KT+KP+NPER+KTH+ITMAX+LENGTH+NWEL	NW.DIML.DIPRN	190
	AMWALNOI . TNOI . R. P. PILIS TALLS TOKI . TOKZ	PRN	200

		COMMON /PR/ XLABEL(3), YLABEL(6), TITLE(5), XN1, MESUR, PRNT(122), BLANK	PRN	210
	1	(60) • DIGIT(122) • VF1(6) • VF2(6) • VF3(7) • XSCALE • DINCH • SYM(17) • XN(100) •	PRN	220
	Z	2YN (13) •NA (4) •N1 •N2 •N3 •YSCALE •FACT1 •FACT2 - CONNON: (ADS175 (17, 17, 18, 18, 18, 18, 16, 16, 16, 11, 11, 15, 15, 14, 1843, 1841)	DON	240
		CUMMUN /ARSIZE/ IZ#JZ#IF#JF#IR#JR#IC#JC#IE#JC#IE#JC#IE#JC#IE#AA#IMAA#IMAA	PRN	250
с			PRN	260
č			PRN	270
Ċ		INITIALIZE VARIABLES FOR PLOT	PRN	280
С		***	PRN	290
•		ENTRY MAP	DDN	310
Ç	10		PRN	320
	10	YSE=DINCH#YSCALE	PRN	330
		NYD=YDIM/YSF	PRN	340
		IF (NYD*YSF.LE.YDIM-DELY(INO1)/2.) NYD=NYD+1	PRN	350
		IF (NYD.LE.12) GO TO 20	PRN	360
		DINCH=YDIM/(12.*YSCALE)		3/0
		WRITE (P+310) DINCH TE (MCCALE T 1 0) WRITE (D-330)	DDN	300
		$\frac{1}{10}$	PRN	400
	20	NXD=WIDTH/XSF	PRN	410
		IF (NXD*XSF.LE.WIDTH-DELX(JNO1)/2.) NXD=NXD+1	PRN	420
		N4=NXD*N1+1	PRN	430
		N5=NXD+1	PRN	440
			DON	430
			PRN	470
		NA(1) = N4/2	PRN	480
		NA (3) =N4/2+3	PRN	490
		NC=(N3-N8-10)/2	PRN	500
		ND=NC+N8	PRN	510
		NE=MAX0(N5+N6)	PRN	520
		VF1(3)=DIGIT(ND)	DON	530
		VF2(3)=DIGIT(ND) VF2(3)=DIGIT(NC)	PRN	550
		XI ARFI (3) = MFSUR	PRN	560
		YLABEL (6) = MESUR	PRN	570
		DO 40 I=1.NE	PRN	580
		NNX=N5+I	PRN	590
		NNY=1-1	PRN	610
		IF (NNY GENNO) GU IU JU	PRN	620
	30	TN (1) #TSF "NNT/TSCALL TE (NNX.IT.O) GO TO 40	PRN	630
	50	XN (I) = XSF * NNX/YSCALE	PRN	640
	40	CONTINUE	PRN	650
		RETURN	PRN	660
С			PRN	670
ç			DON	600
С			PAN	700
c			PRN	710
č		VARIABLES INITIALIZED EACH TIME A PLOT IS REQUESTED	PRN	720
		DIST=WIDTH-DELX(JNO1)/2.	PRN	730
		10NL≖LL	PRN	740
			DON PRN	750
		Z#NAU#ADF 16 (NG.FG.1) WDITE (P.280) (TITEF(T).T±1.2)	PRN	770
		IF (NG_EG_2) WRITE (P+280) (TITLE(1)+1=3+5)	PRN	780
		DO 270 I=1,N4	PRN	790
С			PRN	800
С		LOCATE X AXES	PBN	910

		IF (I.EG.1.0R.I.EG.N4) GO TO 50	PRN 820
		PRNT(1)=SYM(12)	PRN 830
		PRNT (N8)=SYM(12)	PRN 840
		IF ((I-1)/N1*N1.NE.I-1) GO TO 70	PRN 850
		PRNT(1)=SYM(14)	PRN 860
		PRNT (N8) = SYM (14)	PRN 870
		GQ TQ 70	PRN 880
С			PRN 890
č		OCATE Y AXES	PRN 900
•	50		PRN 910
		$TF_{(1)=1}/N24N2_{0}FR_{0,1}=1$ PRNT(.1)=SYN(14)	PRN 920
	60	IF ((J=1)/N2*N2.NF.J=1) PRNT(J) = SYM(13)	PRN 930
r	00		PPN 940
č			DDN 050
C	70	TE (DIST.IT & OB.DIST.IT 7-YNIAYSE) 60 TO 220	PPN 950
	10		PRN 900
			PRN 910
			DEN GOA
			PPN1000
			PRN1000
		IF (S(L,JJ),LT.0.) GO TO 190	PRNIUIU
		INDX3=0	PRN1020
		GO TO (80,90), NG	PRN1030
	80	K=(SURI(L,JJ)-PHI(L,JJ))*FACT1	PRN1040
С		-TO CYCLE SYMBOLS FOR DRAWDOWN, REMOVE C FROM COL. 1 OF NEXT CARD-	PRN1050
С		K=AMOD(K+10+)	PRN1060
		GO TO 100	PRN1070
	90	K=PHI(L,J)+FACT2	PRN1080
1	00	IF (K) 110,140,120	PRN1090
1	10	IF (J-2.GT.0) PRNT(J-2)=SYM(13)	PRN1100
		N=-K	PRN1110
		IF (N.LT.100) GO TO 130	PRN1120
		GO TO 170	PRN1130
1	20	N=K	PRN1140
		IF (N.LT.100) GO TO 130	PRN1150
		IF (N.GT.999) GO TO 170	PRN1160
		INDX3=N/100	PRN1170
		$IF (J=2.GT_0) PRNT(J=2)=SYM(INDX3)$	PRN1180
		N=N-INDX3+100	PRN1190
1	30	INDX1=MCD(N+10)	PRN1200
		IF (INDX1.EQ.0) INDX1=10	PRN1210
č		-TO CYCLE SYMBOLS FOR DRAWDOWN, REMOVE C FROM COL. 1 OF NEXT CARD-	PHN1220
C		IF $(NG \cdot EG \cdot 1)$ go to 150	PRN1230
		INDX2=N/10	PRN1240
		IF (INDX2.GT.0) GO TO 160	PRN1250
		INDX2=10	PRN1260
		IF (INDX3.EQ.0) INDX2=15	PRN1270
		GO TO 160	PRN1280
1	40	INDX1=15	PRN1290
1	50	INDX2=15	PRN1300
1	60	IF $(J-1,GT,0)$ PRNT $(J-1)=SYM(INDX2)$	PRN1310
		PRNT(J)=SYM(INDX1)	PRN1320
		GO TO 200	PRN1330
1	70	DO 180 II=1,3	PRN1340
		JI=J=3+II	PRN1350
1	80	IF (JI.GT.0) PRNT(JI)=SYM(11)	PRN1360
1	90	IF (S(L+JJ)+LT+0+) PRNT(J)=SYM(16)	PRN1370
2	00	YLEN=YLEN+(DELY(L)+DELY(L+1))/2.	PRN1380
2	10	DIST=DIST=(DELX(JJ)+DELX(JJ=1))/2.	PRN1390
		JJ=JJ - 1	PRN1400
		IF (JJ.EQ.0) GO TO 220	PRN1410
		IF (DIST.GT.Z-XN1#XSF) GO TO 210	PRN1420

	220	CONTINUE	PRN	1430
ç			PRN	1440
С		PRINT AXES,LABELS, AND SYMBOLS	PRN	1450
		IF (I-NA(LL).EQ.0) GO TO 240	PRN	1460
		IF ((I=1)/NI=NI=(I=1)) 250,230,250	PRN	1470
	230	WRITE (P,VF1) (BLANK(J),J=1,NC), (PRNT(J),J=1,NB),XN(1+(I+1)/6)	PRN	1480
		GO 10 260	PRN	1490
	240	WRITE (P+VF2) (BLANK(J)+J=1+NC)+(PRNT(J)+J=1+NB)+XLABEL(LL)	PRN	1500
		↓↓=↓↓ + 1	PRN	510
		GO TO 260	PRN	520
	250	WRITE (P+VF2) (BLANK(J)+J=1+NC)+(PRNT(J)+J=1+N8)	PRN	530
С			PRN	540
С		COMPUTE NEW VALUE FOR Z AND INITIALIZE PRNT	PRN	550
	5e0	Z=Z-2.4×N14×SF	PRN	560
		D0 270 J=1•N8	PRN	570
	270	PRNT(J)=SYM(15)	PRN)	580
С			PRNI	590
С		NUMBER AND LABEL Y AXIS AND PRINT LEGEND	PRN	600
		WRITE (P+VF3) (BLANK(J)+J=1+NC)+(YN(I)+I=1+N6)	PRN	610
		WRITE (P+300) (YLABEL(I)+I≠1+6)	PRN	620
		IF (NG.EQ.1) WRITE (P.290) FACT1	PRN]	630
		IF (NG.EQ.2) WRITE (P.290) FACT2	PRNI	640
		RETURN	PRN]	650
ç			PRN1	660
C		FORMATS	PRN1	670
C			PRN1	680
C			PRNI	690
C			PRN1	700
C			PRN1	710
	200		PRN1	720
	270	FORMAL ("DEAPLANATION") TOTI ("=")// R = CONSTANT HEAD BOUNDARY /	PRNI	730
	200	FORMAT (10, 200, 440)	PRNI	740
	300	FURMAL (10/13741040) Format (10/13741040) Format (10/13741040)	PHNI	750
	310	FO TO - GIE 7.19, 10/4411	PHNI	780
	320	$\frac{1}{10} \frac{1}{10} \frac$	PRNI	770
	520	END	PON1	700
				190-
			D (D	10
c				10
C		REAL #AXIABEL VIABELATITIEXXIAMESURARHOADADAEAH		20
				40
	1	CONTRAL FAK ARECHASTPADT		50
с	•		RLD	60
-		COMMON /DPARAM/ RHO,B,D,F,H	BID	70
		COMMON /SARRAY/ VF4(11), CHK(15)	BLD	80
		COMMON /SPARAM/ WATER, CONVRT, EVAP, CHCK, PNCH, NUM, HEAD, CONTR, FROR .I E	BLD	90
	1	AK, RECH, SIP, U, SS, TT, TMIN, ETDIST, QET, ERR, TMAX, CDLT, HMAX, YDIM, WIDTH.	BLD	100
	2	NUMS+LSOR+ADI+DELT+SUM+SUMP+SUBS+STORE+TEST+ETQB+ETQD+FACTX+FACTY+	BLD	110
	3	IIERR,KOUNT,IFINAL,NUMT,KT,KP,NPER,KTH,ITMAX,LENGTH,NWEL,NW,DIML,DI	BLD	120

COMMON /PR/ XLABEL(3), YLABEL(6), TITLE(5), XN1, MESUR, PRNT(122), BLANKBLD 140 1(60), DIGIT(122), VF1(6), VF2(6), VF3(7), XSCALE, DINCH, SYM(17), XN(100), BLD 150

DATA CHK/ PUNC +, + WATE +, + CONT +, + NUME +, + CHEC +, + EVAP +, + CONV +, + HEAD +, + BLD 210

COMMON /ARSIZE/ IZ, JZ, IP, JP, IR, JR, IC, JC, IL, JL, IS, JS, IH, IMAX, IMX1

DATA IZ+JZ+IP+JP+IR+JR+IC+JC+IL+JL+IS+JS+IMAX/13+20/+IH/1/

с с 4MW.JNO1.INO1.R.P.PU.I.J.IDK1.IDK2

2YN(13) +NA(4) +N] +N2+N3+YSCALE+FACT1+FACT2

BLD 130

BLD 160

BLD 170

BLD 190

BLD 200

1LEAK'++'RECH'++'SIP ++'LSOR'+,'ADI'++'DK1 ++'DK2 +/+R+P+PU/5+6+7/+B+D+BLD	220
2F+H/4#0.D0/ BLD	230
DATA SYM/111+12++13++4++15++16++17++18++19++10++++++++++++++++++++++++++++	240
1 ','R','W'/ BLD	250
DATA PRNT/122** */*******************************	260
1 */•NA(4)/1000/ BLD	270
DATA XLABEL/! X DIS- ", TANCE IN', MILES '/, YLABEL/ DISTANCE', BLD	280
1FROM OR ++ IGIN IN ++ Y DIRECT ++ ION+ IN ++ MILES +/+TITLE/PLOT BLD	290
20F !, 'DRAWDCWN', 'PLOT OF ', 'HYDRAULI', 'C HEAD'/ BLD	300
DATA DIGIT/11++2++3++4++5++6++7++8++9++10++11+++12++13+BLD	310
1, 14', 15', 16', 17', 18', 19', 20', 21', 22', 23', 24', 25', 26', BLD	320
2+27+,+28+,+29+,+30+,+31+,+32+,+33+,+34+,+35+,+36+,+37+,+38+,+39+,+BLD	330
340 · • · 41 · • · 42 · • • 43 · • · 44 · • · 45 · • · 46 · • · 47 · • · 48 · • · 49 · • · 50 · • · 51 · • · 52 · • · 5BLD	340
431, 1541, 1551, 1561, 1571, 1581, 1591, 1601, 1611, 1621, 1631, 1641, 1651, 166BLD	350
5+,+67+,+68+,+69+,+70+,+71+,+72+,+73+,+74+,+75+,+76+,+77+,+78+,+ 79BLD	360
6', '80', '81', '82', '83', '84', '85', '86', '87', '88', '89', '90', '91', '92'BLD	370
7, +93+, +94+, +95+, +96+, +97+, +98+, +99+, +100+, +101+, +102+, +103+, + 104+BLD	380
8+105++106++107++108++109++110++111++112++113++114++115*BLD	390
9++116+++117+++118+++119+++120+++121+++122+/ BLD	400
DATA VF1/1(1H +++++++++++++++++++++++++++++++++++	410
DATA VF2/1(1H +++++++++++++++++++++++++++++++++++	420
DATA VF3/*(1H0*+*+*** *+*A1+F*+*3.1+*+*12F1*+*0.2)*/ BLO	430
DATA VF4/+(1H0+++++++ +++++++++++++++++++++++++++++	440
10'+'F6-1'+'))'/ BLD	450
	460
END BLD	470-

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